

RESOLUTION 2024-3

**A RESOLUTION APPROVING SITE PLAN FOR
STOW & GO STORAGE, LLC, AND PROVIDING TERMS AND
CONDITIONS OF APPROVAL**

WHEREAS, Stow & Go Storage, LLC (“Applicant”), has applied for site plan approval for a site plan on the following described properties:

a. Parcel ID (Parcel 1): 1433-123-0006-000-3;
Legal Description of Property: 33 34 40 BEG AT PT WHERE S BDRY OLSONS S/D INT E RD R/W LI OF HWY US 1, TH N 89 25 19 E 400.80 FT, TH S 01 23 41 E 245.29 FT, TH S 89 29 08 W 320.61 FT TO E RD R/W LI OF HWY US 1, TH NWLY ON CURVE CONC NE, R OF 11, 359.2 TH NWLY ALG ARC 258.82 FT TO POB (2.04 AC)

b. Parcel ID (Parcel 2): 1433-124-0005-010-2;
Legal Description of Property: 33 34 40 FROM INT OF S BDRY OLSONS S/D AND E RD R/W LI OF HWY US 1 AND CURVE CONC NE, R OF 11,359.2 FT, TH SELY ALG ARC 258.82 FT TO POB; TH CONT ALG ARC 110.64 FT, TH N 89 29 08 E 268.08 FT, TH S 21 31 34 E 228.78 FT, TH N 68 28 26 E 125 FT, TH N 21 31 34 W 407.92 FT, TH S 89 25 19 W 39.78 FT, TH S 01 23 41 E 108.33 FT, TH S 89 29 08 W 320.61 FT TO POB (1.69 AC)

c. Parcel ID (Parcel 3): 1433-124-0005-020-5;
Legal Description of Property: 33 34 40 FROM INT OF S BDRY LI OLSON’S S/D WITH ELY R/W LI US #1 RUN SELY ALG ELY R/W 369.46 FT TO POB, TH CONT SELY ALG SD ELY R/W 240 FT, TH N 87 DEG 09 MIN 04 SEC E 265.12 FT, TH N 21 DEG 31 MIN 34 SEC W 228.78 FT, TH S 89 DEG 29 MIN 08 SEC W 268.08 FT TO POB (1.35 AC)

Total Size of Property (Parcel 1, 2, 3): 5.08 Acres

WHEREAS, the site plan is submitted for the permitted use, Outdoor Storage Facilities for Families and Businesses, which is permitted subject to site plan approval in the commercial zoning district, in which the properties are located.

WHEREAS, the Applicant, in its submittal, has described the types of vehicles to be stored at the subject facility as:

The site is considered an Outdoor Open–Air Storage business. The items being proposed for storage vary ranging from boats on trailers, recreational vehicles including travel trailers, motorhomes, and accessory trailers. All units stored, will be required to be portable.

WHEREAS, Applicants submittal also provided that, “no semi-trucks or trailers will be stored on this property” and the Applicant had previously described the desired storage to be boat and RV storage.

WHEREAS, the entrance/exit gate at the north end of the property cannot be used in accordance with FDOT regulations without reconfiguration but Applicant desires to maintain the entrance for emergency use.

WHEREAS, the Applicant’s plan is based on access through the south gate which will be a manned gate that will be opened and closed each day and,

WHEREAS, Applicant has submitted, and the Village engineer has reviewed and approved as in compliance, the site plan submitted as attached Exhibit A.

WHEREAS, the Applicant has submitted the landscape plan attached as Exhibit B which was deemed in compliance by the Village landscape architect.

WHEREAS, the Applicant has secured a variance, a copy of which is attached as Exhibit C, and submitted the revised landscape plan attached as Exhibit D which meets the requirements of the code after the variance relief.

WHEREAS, the Applicant has submitted construction plans copies attached as Exhibit E.

WHEREAS, the Applicant has submitted irrigation plans copies attached as Exhibit F.

WHEREAS, the Applicant has submitted drainage calculations copy attached as Exhibit G.

NOW, THEREFORE, BE IT RESOLVED BY THE BOARD OF ALDERMEN OF THE TOWN OF ST. LUCIE VILLAGE, FLORIDA, that:

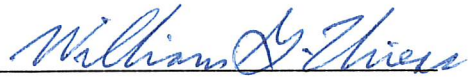
1. The plans and submittals attached as Exhibits A and D – G are approved, subject to the terms and conditions herein.
2. No changes to the approved plans or calculations shall be made without submittal to the Board of Aldermen of the Town of St. Lucie Village for review and approval.
3. This site plan approval is for the entire site, that is for the three parcels which make up the site, together, and not separately.
4. Applicant, having commenced work adjacent to the St. Lucie School property and having encroached onto the St. Lucie School property, shall sod and irrigate and maintain the property until ground cover is securely established.
5. In addition, any work, including sloping shall be removed from the St. Lucie School property so that the entirety of Applicant’s and improvements are located solely on Applicants property and not the Village’s property. In that regard, Applicant’s contractor shall coordinate with the Village’s surveyor to confirm the boundary line around the St. Lucie School and shall construct the project accordingly.

6. Upon completion of the proposed paving, grading and drainage improvements, the engineer of record shall provide final certification to the Town of St. Lucie Village , upon completion of the project that the project or system and its components, as approved hereby, have been constructed in substantial conformance with the approved plans, including certified "as-builts" completed by a Florida registered land surveyor.
7. Applicant and any successors in interest shall be responsible for maintaining in good order and repair the improvements, including the swales and retention areas, trees and shrubberies, the opaque green screen fence, and the like, which responsibility shall include repair and replacement of the same, if necessary; and, in particular, dead trees, shrubs, and sod shall be replaced within 90 days with plants conforming to the approved plan.
8. Applicant shall reimburse the Village's engineer and landscape architect expenses before commencing work.
9. The items stored on the subject property shall be limited to boats on trailers, recreational vehicles including travel trailers, motorhomes, accessory trailers and campers, as well as boat trailers, cars and pick-up trucks, and storage pods.
10. Semi-tractors and trailers, commercial vehicles with more than two axles, and heavy machinery and heavy equipment (except in conjunction with licensed businesses on the Parcel 3 (the parcel at 2450 N. US 1) and stored on that property only) are not permitted. Also prohibited are junk vehicles or items that may be stored in a junk yard, yard waste or other debris, unserviceable vehicles, or cargo containers.
11. While the northern emergency access may be maintained, its use shall be limited to emergencies and project access shall be limited to the south gate at 2450 N US 1, which shall be a manned gate that is opened and closed each day. Any deviation from the stated entry gate operation should be reviewed by FDOT and submitted as a modification of the site plan approval.
12. The applicant shall provide irrigation for the new landscaping south of that depicted on Exhibit F pursuant to a plan designed by a qualified professional, and the applicant's design professional shall approve and provide the plan to St. Lucie Village and shall, in the certification referenced at paragraph 6 above, reference completion of those improvements in accordance with that plan.


(Signature page and attestation follow)

PASSED AND APPROVED by the Board of Aldermen of the Town of St. Lucie Village on
the ^{16th}24th day of April, 2024.

APPROVED:
BOARD OF ALDERMEN OF THE TOWN OF
ST. LUCIE VILLAGE, FLORIDA

By: 
William G. Thiess, Mayor

ATTESTED:

By: 
Paulette T. Burgess, Acting Clerk

I, Paulette T. Burgess, Acting Clerk of the TOWN OF ST. LUCIE VILLAGE, FLORIDA, do
hereby certify that this is a true and accurate copy of Resolution **2024-3** which was duly
introduced, read, and adopted at the regular meeting of the Board of Aldermen of the TOWN
OF ST. LUCIE VILLAGE, FLORIDA, held the ^{16th} day of April, 2024.


Paulette T. Burgess, Acting Clerk

**IN RE: PETITION FOR VARIANCE OF
STOW & GO, LLC**

ORDER

This matter came before the Board of Adjustment of the Town of St. Lucie Village, Florida, on March 20, 2024, at 6:30 p.m. at the Village Hall on the request of Stow & Go, LLC for relief from provisions of the St. Lucie Village Land Development Code applicable to its property.

The Board of Adjustment, having considered the petition of Stow & Go, LLC, having heard from Brandon and Meghan Haynes, Dylan O'Berry, PE, and the public, and having otherwise given the matter full consideration, finds as follows:

1. The hearing was properly noticed by publication and mailed notices, as required by the St. Lucie Village Land Development Code.

2. The subject property is three contiguous properties, zoned commercial, currently improved with pavement and fencing, located at 2500 N US Hwy 1, Fort Pierce, Florida 34946, and the tax I.D. number and legal description is as follows:

a. Parcel ID (Parcel 1): 1433-123-0006-000-3;

Legal Description of Property: 33 34 40 BEG AT PT WHERE S BDRY OLSONS S/D INT E RD R/W LI OF HWY US 1, TH N 89 25 19 E 400.80 FT, TH S 01 23 41 E 245.29 FT, TH S 89 29 08 W 320.61 FT TO E RD R/W LI OF HWY US 1, TH NWLY ON CURVE CONC NE, R OF 11, 359.2 TH NWLY ALG ARC 258.82 FT TO POB (2.04 AC)

b. Parcel ID (Parcel 2): 1433-124-0005-010-2;

Legal Description of Property: 33 34 40 FROM INT OF S BDRY OLSONS S/D AND E RD R/W LI OF HWY US 1 AND CURVE CONC NE, R OF 11,359.2 FT, TH SELY ALG ARC 258.82 FT TO POB; TH CONT ALG ARC 110.64 FT, TH N 89 29 08 E 268.08 FT, TH S 21 31 34 E 228.78 FT, TH N 68 28 26 E 125 FT, TH N 21 31 34 W 407.92 FT, TH S 89 25 19 W 39.78 FT, TH S 01 23 41 E 108.33 FT, TH S 89 29 08 W 320.61 FT TO POB (1.69 AC)

Exc

c. Parcel ID (Parcel 3): 1433-124-0005-020-5;

Legal Description of Property: 33 34 40 FROM INT OF S BDRY LI OLSON'S S/D WITH ELY R/W LI US #1 RUN SELY ALG ELY R/W 369.46 FT TO POB, TH CONT SELY ALG SD ELY R/W 240 FT, TH N 87 DEG 09 MIN 04 SEC E 265.12 FT, TH N 21 DEG 31 MIN 34 SEC W 228.78 FT, THS 89 DEG 29 MIN 08 SEC W 268.08 FT TO POB (1.35 AC)

Total Size of Property (Parcel 1, 2, 3): 5.08 Acres

3. Stow & Go Storage, LLC requests five variances for relief from Ordinance 2012-4 (now incorporated into the St. Lucie Village Land Development Code at Section 3.6.1, et seq.), particularly,

- a. From Section 3.6.4(B)(2)(a) Perimeter Landscaping Relating to Abutting Properties, requiring a 42" planting height in a seven-gallon container, so as to allow a planting height of 18" in a three-gallon container,
- b. From Section 3.6.4(B)(2)(b) Perimeter Landscaping Relating to Abutting Properties, requiring a tree be provided or preserved for each 30 linear feet or major fraction thereof, so as to provide relief from the requirement along a portion of the north perimeter of the property, as shown in the attached plan provided that the total tree count required for that portion of the north perimeter will remain on the property;
- c. From Section 3.6.4(B)(2)(c) Perimeter Landscaping Relating to Abutting Properties (Proposed Parking Abutting Existing Barrier), requiring that when a proposed parking area abuts an existing hedge wall or durable landscape barrier on an abutting property, so that the landscape buffer and tree count requirements are not required to be included in the landscape plan on the east side of the property, as shown on the attached plan;
- d. From Section 3.6.4(B)(3) Parking area interior landscaping, so as to eliminate the requirement for interior landscaping; and

- e. From Section 3.6.4(2), Planting requirements (tree count) so that the total tree count (of new/additional trees) to be added to the site is 37 rather than 72.
- 4. Concerning the variance requests described at paragraph 3(a) above, the standards at Section 9.2.0, including particularly any hardship beyond cost, have not been met.
- 5. The variance requests described at paragraphs 3(b) through 3(e) (“those requests”) arise from conditions that are unique to the land or structures involved.
- 6. As to those requests, literal enforcement of the Code would deprive Property Owners of rights commonly enjoyed by other property owners in the same zoning district.
- 7. Granting those requests as specified herein will not be materially detrimental or injurious to other property or improvements in the neighborhood.
- 8. Granting the below variances will not confer any special privilege to Property Owners which is denied to owners of similar properties.
- 9. The variances set forth below are the minimum variance which will make possible the reasonable use.
- 10. The spirit and intent of the Zoning Ordinance and Town Comprehensive Plan are not opposed to the specified variance.

It is thereupon,

ORDERED as follows:

- 1. The Petition for relief from Section 3.6.4(B)(2)(a) Perimeter Landscaping Relating to Abutting Properties is DENIED.
- 2. The requested variance is GRANTED so that Property Owners are granted relief from Section 3.6.4(B)(2)(b), that is from the requirement for landscaping along the west portion of the north perimeter of the property, as shown in the attached plan, provided, however, that the total tree count required for that portion of the north perimeter will remain on the property.

3. The requested variance is GRANTED so that Property Owners are granted relief from Section 3.6.4(B)(2)(c), so that the landscape buffer and tree count requirements are not required to be met on the east side of the property extending south from the St. Lucie School, as shown on the attached plan.

4. The requested variance is granted so that Property Owners are granted relief from Section 3.6.4(B)(3), so as to eliminate the requirement for interior landscaping.

5. The requested variance is granted so that Property Owners are granted relief from Section 3.6.4(2) so that the total tree count (of new/additional trees) to be added to the site is 37 rather than 72.

6. Property Owners are not otherwise granted relief from the provisions of the Ordinance, the applicable building code(s), or any permit requirement(s).

7. This variance expires automatically in twelve (12) months unless a permit has been issued for the proposed improvement(s).

DONE AND ORDERED by the Board of Adjustment of the Town of St. Lucie Village, Florida, this ____ day of April, 2024.

BOARD OF ADJUSTMENT TOWN OF
ST. LUCIE VILLAGE, FLORIDA

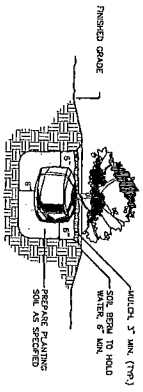
By: _____
Jim Van Hekken, Chair

Copies furnished to:

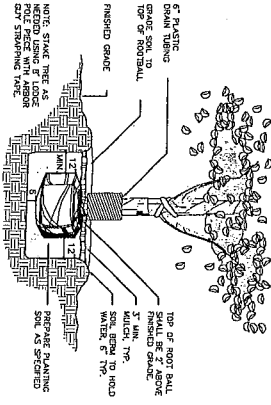
Stow & Go, LLC
Board of Adjustment
William G. Thiess, Mayor

Board of Aldermen
Paulette T. Burgess, Acting Clerk
Carl Peterson, Building Official

SHRUB PLANTING DETAIL



TREE PLANTING DETAIL



TREE CALCULATIONS

NORTH WEST PL 370 LF - TOTAL 57 ACCESS WAY=	315 LF
EAST PL=	386 LF
NORTH PL=	138 LF
EAST R. (OPEN) SCREEN ADJUSTING PROP. LANDSCAPE BARRIER=	39 LF
SOUTH PL (OPEN) SCREEN ADJUSTING PROP. CONCRETE WALL=	0 LF
TOTAL=	878 LF / 30

TREES PERIMETER REQUIRED=	29
INTERIOR PARKING ISLAND TREES=	22
EXISTING TREE CROUNTS=	(14)
TOTAL TREES REQUIRED=	37
TOTAL TREE PROVIDED=	37

LANDSCAPE CONTRACTOR NOTES

ALL PLANT MATERIAL SHALL BE FURNISHED BY THE LANDSCAPE CONTRACTOR. THOSE PLANTS THAT DO NOT MEET THE SPECIFICATIONS IN SIZE OR GRADE WILL BE REJECTED.

THE LANDSCAPE CONTRACTOR SHALL HAVE A COMPETENT FOREIGN IDENTIFIED AT THE BEGINNING OF THE JOB WHO WILL BE ON THE JOB THROUGHOUT THE INSTALLATION OF PLANT MATERIAL. THE PERSON SHALL BE RESPONSIBLE FOR DIRECTING THE CREW AND ANY SUBCONTRACTORS WORKING FOR THE LANDSCAPE CONTRACTOR AND SHALL ACT AS THE LANDSCAPE CONTRACTOR'S REPRESENTATIVE TO THE OWNER AND LANDSCAPE ARCHITECT.

THE LANDSCAPE CONTRACTOR SHALL LAY OUT AREAS TO BE PLANTED AND ARRANGE A MEETING WITH THE LANDSCAPE ARCHITECT FOR APPROVAL BEFORE PLANTING.

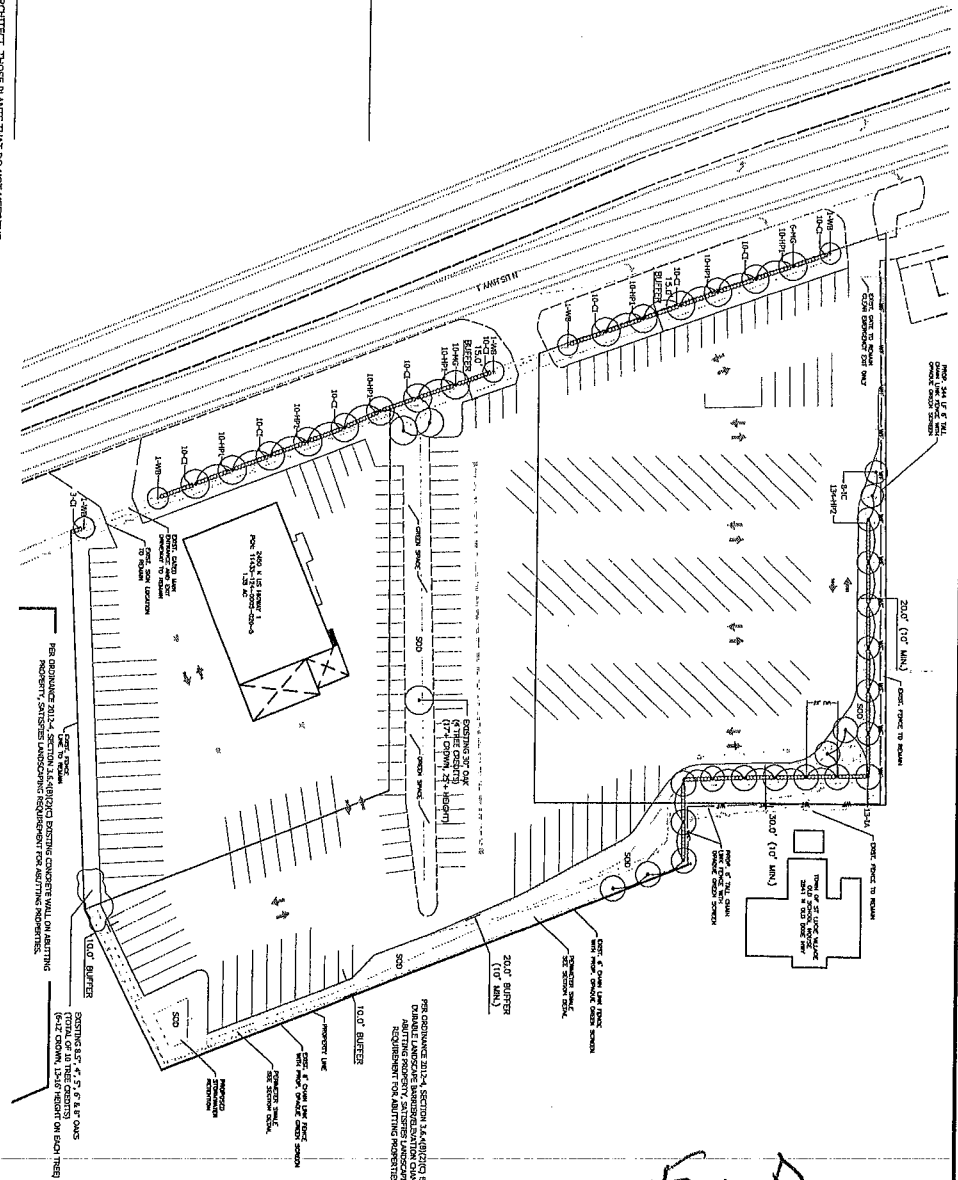
THE LANDSCAPE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PREPARATION OF ALL AREAS TO RECEIVE PLANTS. ALL PLANTS, GRASSES AND WEEDS SHALL BE REMOVED BY HAND, ROTOTILLING OR BY SCARPING.

WITH WATER PRIOR TO PLANTING. IF WATER DOES NOT COMPLETELY PERMEATE FROM HOLES WITHIN 4 HOURS, CONTACT THE LANDSCAPE ARCHITECT FOR APPROVAL BEFORE PLANTING. PLANT HOLES ARE TO BE BACKFILLED WITH HOMOGENEOUS SOIL, NOT CONSISTING OF 2 PARTS 5/8" SAND, 1 PART FERTILIZER AND 1 PART COMPOST OR MEDIUM GRADE. BACKFILL SHALL BE ADDED TO EACH PLANT HOLE ACCORDING TO THE FOLLOWING SPECIFICATIONS:

RECOMMENDED APPLICATION RATES: BACKFILL HALFWAY UP THE ROOTBALL, PLACE THE TABLETS BESIDE THE ROOTBALL ABOUT 1 INCH FROM THE ROOT TIPS. DO NOT PLACE TABLETS IN BOTTOM OF HOLE.

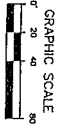
THE CONTRACTOR SHALL PROVIDE THE SPECIFIED MULCH. INSTALL 3" DEPTH OF SPECIFIED MULCH TO ALL AREAS INDICATED ON THE PLAN. THE MULCH SHALL BE INSTALLED AS SHOWN ON THE DETAILS TO ENSURE THAT IT IS NOT INSTALLED TOO CLOSE TO THE PLANT TRUNK OR STEM.

THE LANDSCAPE CONTRACTOR SHALL GUARANTEE THAT ALL PLANT MATERIAL SHALL BE ALIVE AND THRIVING FOR A PERIOD OF ONE YEAR FOLLOWING PLANTING. THE LANDSCAPE CONTRACTOR SHALL REPLACE DEAD OR SICK PLANTS AT NO ADDITIONAL COST TO THE OWNER.



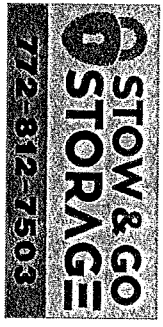
PLANT SCHEDULE

TREES & PALMS	SHRUBS	OTHER MATERIALS
10-01	10-01	10-01
10-02	10-02	10-02
10-03	10-03	10-03
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10-100	10-100	10-100



Ex D

<p>ISSUING SCALE: AS SHOWN</p> <p>PROJECT NUMBER: 24-002</p> <p>SHEET: 1-01</p>	<p>DATE: FEBRUARY 14, 2024</p> <p>DESIGNED BY: [Signature]</p> <p>CHECKED BY: [Signature]</p> <p>PROJECT NUMBER: 24-002</p> <p>ADDITIONAL SHEETS: APRIL 12, 2024</p>	<p>REGISTERED LANDSCAPE ARCHITECT</p> <p>STATE OF FLORIDA</p> <p>NO. 148730</p> <p>LANDSCAPE ARCHITECT</p>	<p>SHEET TITLE:</p> <p>LANDSCAPE PLAN</p>	<p>PROJECT TITLE/LOCATION:</p> <p>STOW & GO STORAGE</p> <p>2450 N US HWY 1</p> <p>FT. PIERCE, FLORIDA</p>	<p>MAGNOLIA LANDSCAPE GROUP</p> <p>1801 W. US HWY 1</p> <p>FT. PIERCE, FLORIDA 34920</p>
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CONSTRUCTION PLANS & SPECIFICATIONS FOR STOW & GO STORAGE TOWN OF SAINT LUCIE VILLAGE, FLORIDA

THESE PLANS HAVE BEEN PREPARED IN ACCORDANCE WITH AND ARE GOVERNED BY THE STATE OF FLORIDA, DEPARTMENT OF TRANSPORTATION, DESIGN STANDARDS, LATEST EDITION (FY 2023-24).

GOVERNING SPECIFICATIONS:
FOOT STANDARD PLANS FOR ROAD AND BRIDGE CONSTRUCTION, LATEST EDITION (FY 2023-24)

THE FLORIDA DEPARTMENT OF TRANSPORTATION, STANDARD SPECIFICATIONS FOR ROAD & BRIDGE CONSTRUCTION, LATEST EDITION (FY 2023-24).

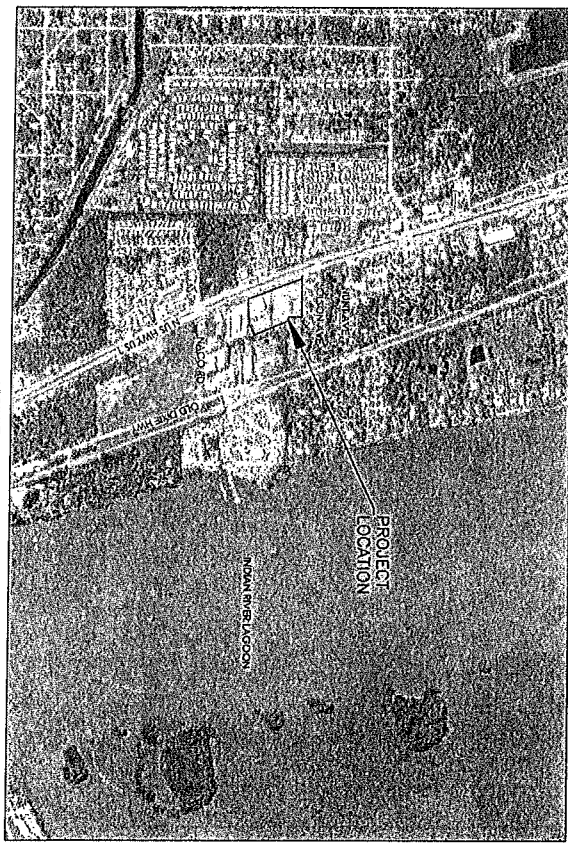
THE MANUAL OF UNIFORM MINIMUM STANDARDS FOR DESIGN, CONSTRUCTION AND MAINTENANCE FOR STREETS AND HIGHWAYS (GREENBOOK), LATEST EDITION (2018).

THE FEDERAL HIGHWAY ADMINISTRATION (FHWA) MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES (M.U.T.C.D.), LATEST REVISION 3, JULY 2022.

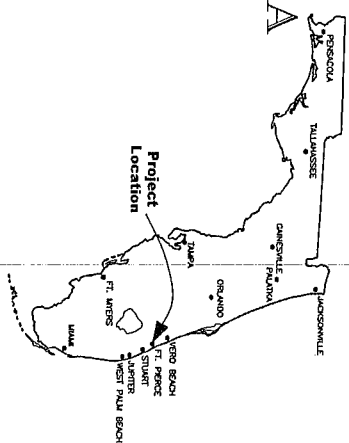
AND THE ST. LUCIE COUNTY LAND DEVELOPMENT REGULATIONS, LATEST EDITION AND STANDARDS SET FORTH BY THE TOWN OF SAINT LUCIE VILLAGE.

CLIENT AND PROPERTY OWNER
BRANDON AND MEGHAN HAYNES
STOW & GO STORAGE
2500 N US HWY-1
FORT PIERCE, FL 34946
(772) 812-7503

CIVIL ENGINEER
GANGSTRANDE ENGINEERING & PLANNING
710 SE OCEAN BLVD
STUART, FL 34994
(772) 888-9076



VICINITY MAP
1" = 800'



SHEET NUMBER	SHEET TITLE
C-1	COVER SHEET
G-1	GENERAL NOTES
S-1	FINAL SITE PLAN
P-1	PAVING, GRADING, & DRAINAGE PLAN
P-2	PAVING, GRADING, & DRAINAGE PLAN
EG-1	STORMWATER POLLUTION PREVENTION PLAN
D-1	DETAILS AND CROSS SECTIONS

JANUARY 2024
FINAL CONSTRUCTION PLANS

REV 3 1/26/24 DO REVISED SECTION B-B PER TOWN COMMENTS
REV 2 8/18/23 DO REVISED FENCING AND SECTIONS PER TOWN COMMENTS
REV 1 6/07/23 DO FINAL SITE PLAN

GEP GANGSTRANDE ENGINEERING AND PLANNING
710 SE OCEAN BLVD
STUART, FLORIDA 34994
PH: (772) 888-9076
WWW.GEP-FL.COM

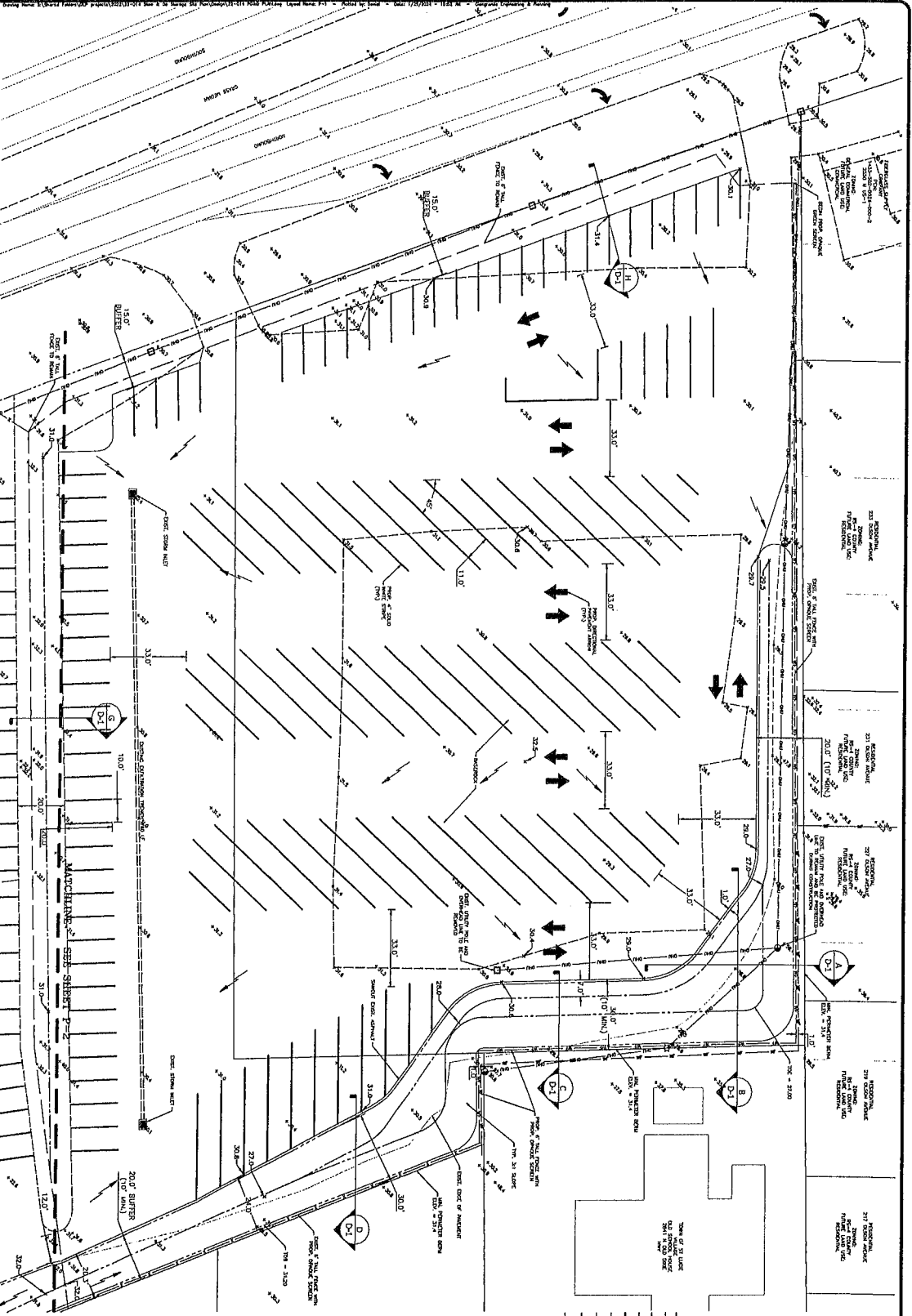
Digitally signed by Arthur D O'Beirne, Jr.
Location: Gainesville, FL, State of FL
Reason: I believe in the accuracy and integrity of this data.
Contact info: email: aob@gep-fl.com or aob@gep-fl.com
Date: 2024.01.19 15:41:50-0500

Scale: 1" = 800'

Sheet: C-1



Requirements Met
Call before posting



NO.	DATE	DESCRIPTION
01	01/15/20	ISSUED FOR PERMITS
02	02/10/20	REVISED TO ADD COMMENTS
03	03/05/20	REVISED TO ADD COMMENTS
04	04/01/20	REVISED TO ADD COMMENTS
05	05/01/20	REVISED TO ADD COMMENTS
06	06/01/20	REVISED TO ADD COMMENTS
07	07/01/20	REVISED TO ADD COMMENTS
08	08/01/20	REVISED TO ADD COMMENTS
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14	02/01/21	REVISED TO ADD COMMENTS
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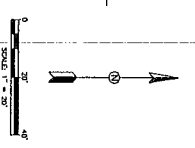
MEGHAN AND BRANDON HAYNES
STOW & GO STORAGE
TOWN OF ST LUCE VILLAGE, FLORIDA

PAVING, GRADING, & DRAINAGE PLAN

FLORIANDE ENGINEERING AND PLANNING
2710 10TH AVENUE
STUART, FLORIDA 34994
PH: (772) 888-9076
FAX: (772) 888-9076
WWW.FEANDP.COM
DATE: 01/15/2020

PROJECT # 22-014
P-1

- LEGEND**
- EXIST. RIGHT OF WAY
 - EXIST. EASE OF PASSENGER
 - EXIST. EASE OF ACCESS
 - PROPERTY BOUNDARY
 - EXIST. DRIVE
 - PROP. DRIVE
 - EXIST. BUILDING
 - PROP. BUILDING
 - EXIST. WOOD FENCE
 - PROP. WOOD FENCE
 - EXIST. FENCE LINE
 - PROP. FENCE LINE
 - DRAINAGE FLOW ARROW
 - PROP. ELEV. POINT
 - EXIST. ELEV. POINT



DATE	8/1	REVISIONS
NO.	001	ISSUED FOR CONSTRUCTION
NO.	002	REVISED TO SHOW MATERIAL QUANTITIES
NO.	003	REVISED TO SHOW EROSION CONTROL MEASURES
NO.	004	REVISED TO SHOW EROSION CONTROL MEASURES
NO.	005	REVISED TO SHOW EROSION CONTROL MEASURES
NO.	006	REVISED TO SHOW EROSION CONTROL MEASURES
NO.	007	REVISED TO SHOW EROSION CONTROL MEASURES
NO.	008	REVISED TO SHOW EROSION CONTROL MEASURES
NO.	009	REVISED TO SHOW EROSION CONTROL MEASURES
NO.	010	REVISED TO SHOW EROSION CONTROL MEASURES

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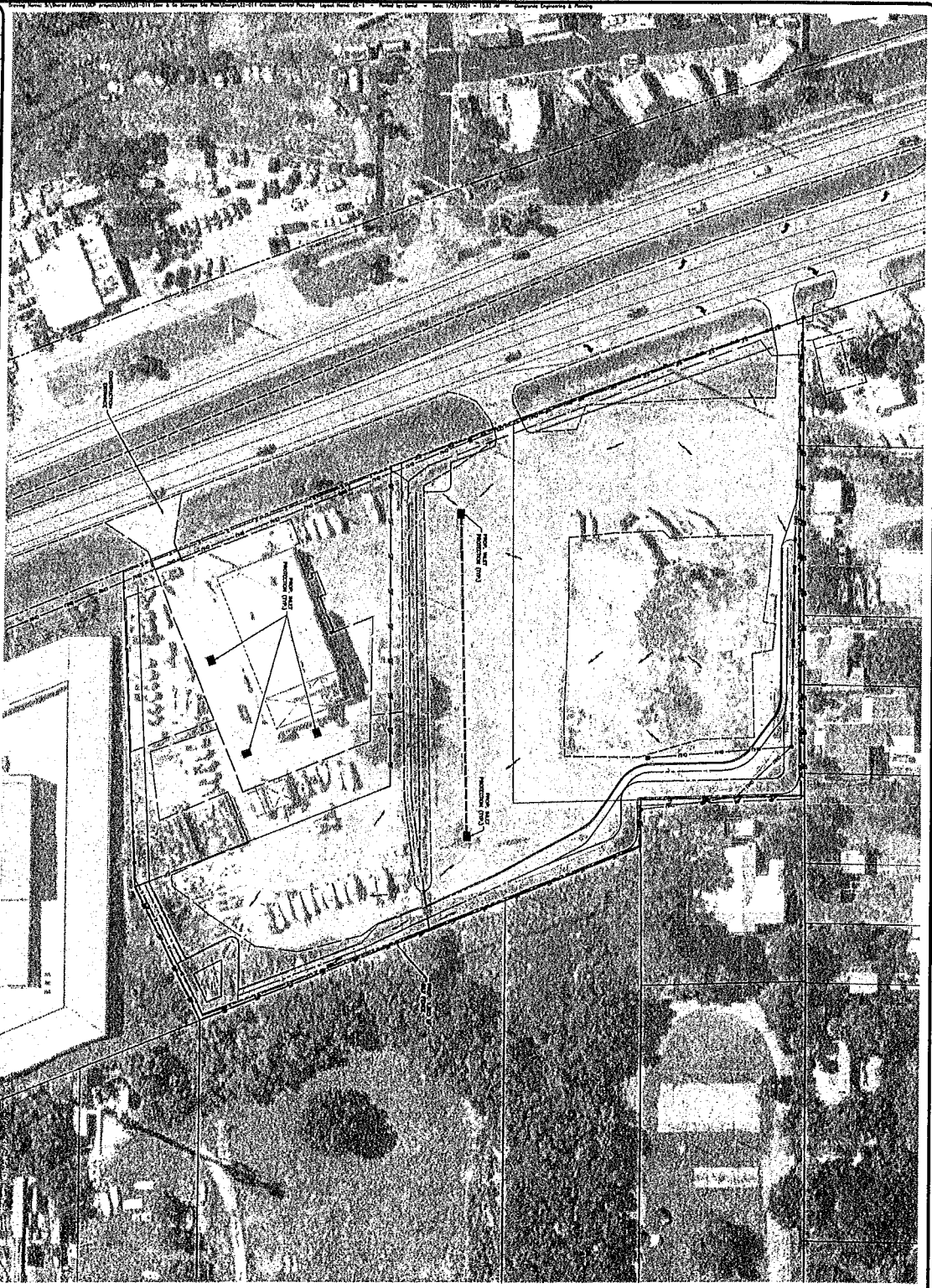
MEGHAN AND BRANDON HAYNES
STOW & GO STORAGE
TOWN OF ST LUCIE VILLAGE, FLORIDA

STORMWATER POLLUTION
PREVENTION PLAN

STANGRADE ENGINEERING AND PLANNING
7700 S.W. 11TH STREET
STUART, FLORIDA 34984
PH: (772) 888-0076
WWW.SEP-FL.COM
Chart No. 200901

Scale: 1" = 40'

PROJECT # 22-014
EC-1



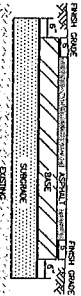
NOTE: REFER TO EROSION CONTROL NOTES
ON SHEET C-1

LEGEND

- PROP. SILT FENCE
- PROP. CHAINLINK SECURITY FENCE
- PROP. SILT FENCE
- PROP. CONTINUOUS BERM

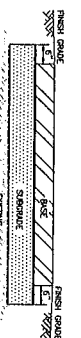


Know what's below.
Call before you dig.



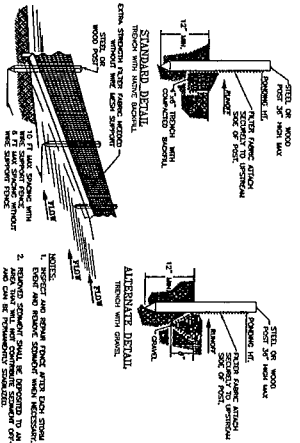
EXISTING TYPICAL PAVEMENT SECTION
SCALE: N.T.S.

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 5" UNCOMPACTED SUBGRADE (US-100) IDENTICAL TO EXISTING
 FINISH GRADE SHALL BE 0.5" ABOVE THE TOP OF THE UNCOMPACTED SUBGRADE AS DETERMINED BY ASTM 1-100 METHOD



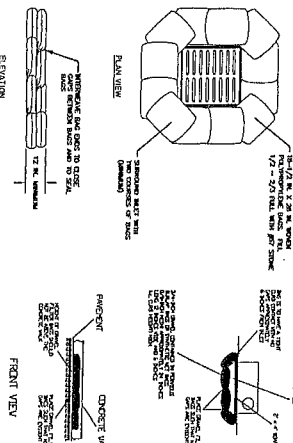
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SCALE: N.T.S.

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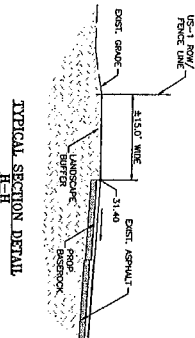


STITCHED RAIL BASS
SCALE: N.T.S.

NOTES:
 1. STITCHED RAIL BASS SHALL BE WOOD POLYPROPYLENE BASS WITH APPROXIMATE
 2. THE BASS SHALL BE 7/8" WIDE BY 2" HIGH
 3. JOINTS OF RAIL BASS USING OTHER BASS STRINGS OR WIRE TIES
 4. RAIL AND THE BASS OF THE BASS ARE STITCHED
 5. A MINIMUM OF 2" WOODING THE BASS WITH A MINIMUM OF TWO (2) JOINTS OF BASS TO A
 6. GRAVEL JITTER BASS INLET PROTECTION DETAIL

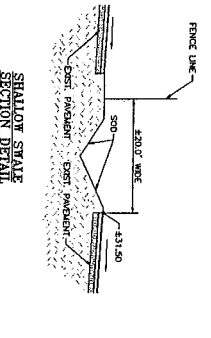


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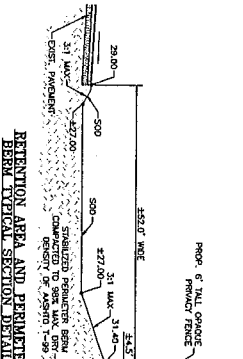
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 5" UNCOMPACTED SUBGRADE (US-100) IDENTICAL TO EXISTING
 FINISH GRADE SHALL BE 0.5" ABOVE THE TOP OF THE UNCOMPACTED SUBGRADE AS DETERMINED BY ASTM 1-100 METHOD



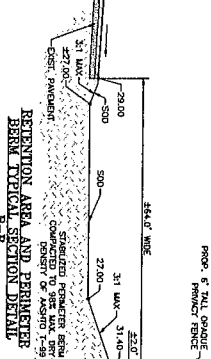
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 FINISH GRADE SHALL BE 0.5" ABOVE THE TOP OF THE UNCOMPACTED SUBGRADE AS DETERMINED BY ASTM 1-100 METHOD



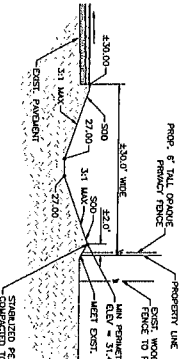
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 FINISH GRADE SHALL BE 0.5" ABOVE THE TOP OF THE UNCOMPACTED SUBGRADE AS DETERMINED BY ASTM 1-100 METHOD



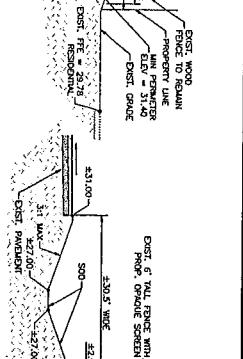
RETENTION AREA AND PERIMETER BERM TYPICAL SECTION DETAIL B-B
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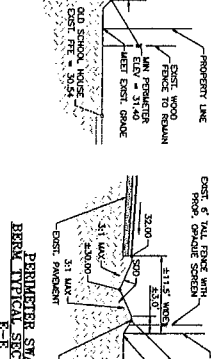
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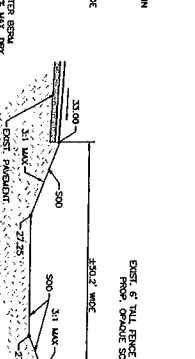
RETENTION AREA AND PERIMETER BERM TYPICAL SECTION DETAIL D-D
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 5" UNCOMPACTED SUBGRADE (US-100) IDENTICAL TO EXISTING
 FINISH GRADE SHALL BE 0.5" ABOVE THE TOP OF THE UNCOMPACTED SUBGRADE AS DETERMINED BY ASTM 1-100 METHOD



PERIMETER SWALE AND BERM TYPICAL SECTION DETAIL E-E
SCALE: N.T.S.

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RETENTION AREA AND PERIMETER BERM TYPICAL SECTION DETAIL F-F
SCALE: N.T.S.

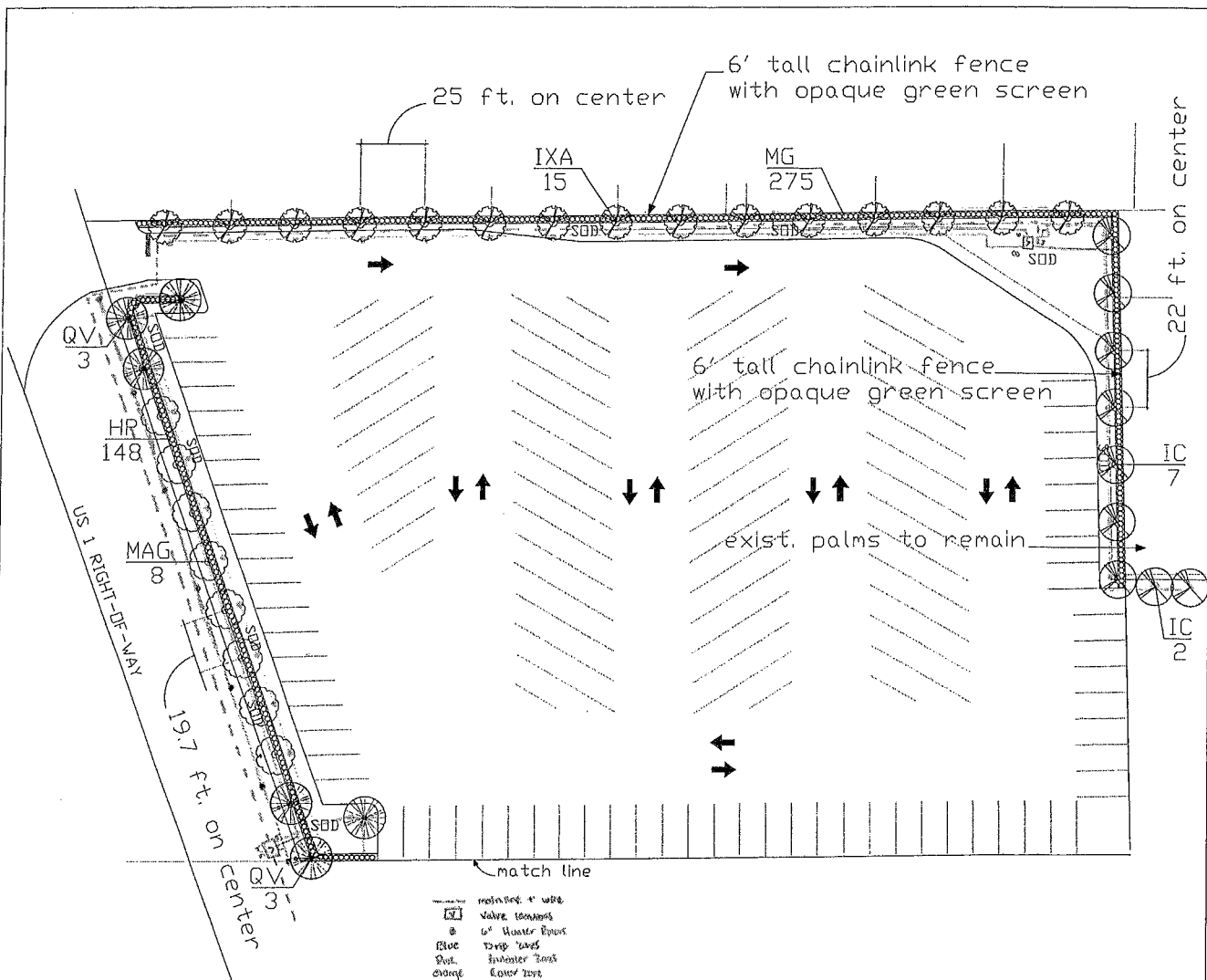
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 5" UNCOMPACTED SUBGRADE (US-100) IDENTICAL TO EXISTING
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MEGHAN AND BRANDON HAYNES
STOW & GO STORAGE
TOWN OF ST LUCIE VILLAGE, FLORIDA

DETAILS AND CROSS SECTIONS

SHARBRAND ENGINEERING AND PLANNING
7000 W. UNIVERSITY BLVD.
SUITE 200
FORT LAUDERDALE, FL 33404
PH: (772) 888-0075
WWW.SHARBRAND.COM

PROJECT # 22-014
D-1



--- material & wire
 [] Valve locations
 & 6" Hunter Riser
 Blue Drop Lines
 Red. Submeter Boxes
 Orange Easement Zone

ST. LUCIE VILLAGE LANDSCAPE ORDINANCE:

ONE TREE FOR EVERY 2500 SQ. FT. OF DEVELOPED AREA 88072 DIVIDED BY 2500 = 35.22 = 36 TREES

4 SPECIES OF TREES REQUIRED FOR 31 TO 40 TREES

4 SPECIES OF TREES PROVIDED TOTALING 36 TREES

2 TREES ADDED EAST OF PROPERTY 2 TREES

TOTAL 38 TREES

PLANT MATERIAL LIST

TREES & PALMS (* indicates native tree)				
Code Name	Quantity	Scientific Name	Common Name	Planting Size
IC	7	ILEX CASSINE	DAHON HOLLY	12" X 3', 4'CT 4" DBH
IXA	17	ILEX ATTENUATA	EAST PALATKA HOLLY	12" X 3', 4'CT 4" DBH
MAG	8	MAGNOLIA GRANDIFLORA	MAGNOLIA	12" X 3', 4'CT 4" DBH
QV	6	QUERCUS VIRGINIANA	LIVE OAK	12" X 3', 4'CT 4" DBH

SHRUBS & GROUNDCOVERS				
Code Name	Quantity	Scientific Name	Common Name	Planting Size
HP	128	HAEMELIA PATENS	FIREBUSH	20" X 18" #3, 24" OC
MG	275	MISCANTHUS GIGANTEUS	GIANT CHINESE SILVER GRASS	18" X 16" #3, 24" OC

OTHER MATERIALS		
MULCH	30	CU. YDS. ENVIRO MULCH (EST.)
SOD	7,600	SQ. FT. BAHIA SOD (EST.)

LANDSCAPE CONTRACTOR NOTES

ALL PLANT MATERIAL SHALL BE FLORIDA #1 OR BETTER. PLANTS WILL BE INSPECTED BY THE LANDSCAPE ARCHITECT. THOSE PLANTS THAT DO NOT MEET THE SPECIFICATIONS IN SIZE OR GRADE WILL BE REJECTED.

THE LANDSCAPE CONTRACTOR SHALL HAVE A COMPETENT PERSON IDENTIFIED AT THE BEGINNING OF THE JOB WHO WILL BE ON THE JOB THROUGHOUT THE INSTALLATION OF PLANTING. THIS PERSON SHALL BE RESPONSIBLE FOR DIRECTING THE CREW AND ANY SUBCONTRACTORS WORKING FOR THE LANDSCAPE CONTRACTOR AND SHALL ACT AS THE LANDSCAPE CONTRACTOR'S REPRESENTATIVE TO THE OWNER AND LANDSCAPE ARCHITECT.

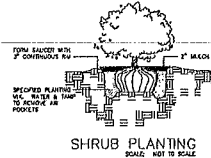
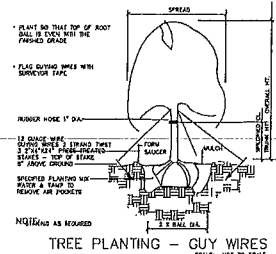
THE LANDSCAPE CONTRACTOR SHALL LAY OUT AREAS TO BE PLANTED AND ARRANGE A MEETING WITH THE LANDSCAPE ARCHITECT FOR APPROVAL BEFORE PLANTING.

THE LANDSCAPE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PREPARATION OF ALL AREAS TO RECEIVE PLANTS. ALL PLANTS, GRASSES AND WEEDS SHALL BE REMOVED BY HAND, ROOTKILLING OR BY SCRAPING.

WITH WATER PRIOR TO PLANTING. IF WATER DOES NOT COMPLETELY DRAIN FROM HOLE WITHIN FOUR HOURS, CONTACT THE LANDSCAPE ARCHITECT FOR APPROVAL. BEFORE PLANTING, PLANT HOLES ARE TO BE BROTTLED WITH A HOMOGENEOUS SOIL MIX CONSISTING OF 2 PARTS SAND, TOPSOIL, 2 PARTS PEAT MOSS AND 1 PART COV MANURE OR MILKINGITE. AGRIFORTH 200-10 PLANTING TABLETS SHALL BE ADDED TO EACH PLANT HOLE. ACCORDING TO THE MANUFACTURER'S RECOMMENDED APPLICATION RATES. BACKFILL HALFWAY UP THE ROOTBALL. PLACE THE TABLETS BESIDE THE ROOTBALL ABOUT 1 INCH FROM THE ROOT TIPS. DO NOT PLACE TABLETS IN BOTTOM OF HOLE.

THE CONTRACTOR SHALL PROVIDE THE SPECIFIED MULCH. INSTALL 3" DEPTH OF SPECIFIED MULCH TO ALL AREAS INDICATED ON THE PLAN. THE MULCH SHALL BE INSTALLED AS SHOWN ON THE DETAILS TO ENSURE THAT IT IS NOT INSTALLED TOO CLOSE TO THE PLANT TRUNK OR STEM.

THE LANDSCAPE CONTRACTOR SHALL GUARANTEE THAT ALL PLANT MATERIAL SHALL BE ALIVE AND THRIVING FOR A PERIOD OF ONE YEAR FOLLOWING PLANTING. THE LANDSCAPE CONTRACTOR SHALL REPLACE DEAD OR SICK PLANTS AT NO ADDITIONAL COST TO THE OWNER.



Ex F

SHEET L-1 OF TWO SCALE: 1" = 20'

LANDSCAPE PLAN
NORTH PORTION
STOW & GO STORAGE

DANIEL J. SUGG
LANDSCAPE ARCHITECT / SITE PLANNER
POST OFFICE BOX 335, JENSEN BEACH, FL 34958
PHONE: (772) 485-1776

DATE: 6/11/22
REV: 8/17/23
REV: 8/21/23
REV: 10/16/23 tree sizes
REV: 12/8/23 add 2 trees east

Stormwater Management Report

Engineering Calculations & Supplementary Documents
Supporting a Surface Water Management System Serving

Stow & Go Storage

Location:
Town of St Lucie Village, FL
St Lucie County, PCN:
1433-123-0006-000-3

Prepared by:

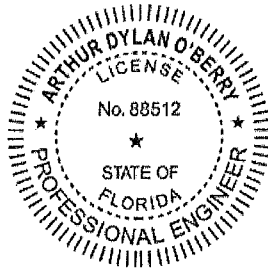


GIANGRANDE
Engineering & Planning
Consulting Civil Engineers
710 SE Ocean Boulevard
Stuart, Florida 34994
772-888-9076
FL Certification: 3093

Revised: December 2023

Signed by:

Dylan O'Berry, PE
PE Reg #88512
Date: 12/12/2023



Digitally signed by Arthur D O'Berry
Location: Giangrande Engineering
and Planning, Stuart, FL
Reason: I am the author of this
document
Contact Info: email:
dylan@gep-lc.com cell: (772)
203-0664
Date: 2023.12.12 14:15:59-05'00'

EXG

Table of Contents

1. Project Description	1
2. Existing Conditions	1
2.1. Existing Site.....	1
2.2. Existing Site Drainage	2
2.3. Control Elevation	2
3. Proposed Conditions	2
3.1. Project Description	2
3.2. Land use	2
3.3. Water Quality	3
3.4. Stormwater Runoff Volume	3
3.5. Stormwater Design Elevation	3

Appendices

Appendix A	Location Map
Appendix B.....	Stormwater Calculations – Existing Conditions
Appendix C.....	Stormwater Calculations – Proposed Conditions
Appendix D	Sub-Basin Map
Appendix E.....	Geotechnical Report

1. PROJECT DESCRIPTION

The Stow & Go Storage facility is owned and operated by Haynes & Son Holdings LLC. It is conveniently situated on the eastern side of US Hwy-1, approximately one mile north of the intersection of US Hwy 1 and SR-A1A. The establishment will be developed utilizing the remnants of a previously demolished building and the existing boat maintenance facility, aiming to provide comprehensive storage solutions for boats, recreational vehicles (RVs), and other similar items. The planned enhancements for the site encompass the refurbishment of the existing asphalt pavement, the utilization shell rock base to stabilize the old building footprint, and the establishment of an on-site stormwater drainage system to ensure water quality and accommodate the full storage capacity of the 100-year, 1-day storm event. Additionally, the site presently incorporates a 260 linear foot exfiltration trench with two inlets.



Image obtained from Google Earth.

Figure 1– Location Map

Location - Section 33, Township 34S, Range 40E, St. Lucie County, FL
27°28'51.51"N, 80°20'19.61"W

2. EXISTING CONDITIONS

2.1. EXISTING SITE

The subject site encompasses 3 parcels of land totaling 5.08 acres of land, situated to the east of US Hwy-1. It benefits from convenient accessibility via three FDOT driveways that connect directly to US Hwy-1. According to historical aeriels and public records, the site was originally developed prior to 1992 and

subsequently subdivided in 2009. In 2007, the building on the premises was demolished, resulting in the presence of the existing asphalt pavement and drainage inlets. For a comprehensive understanding of the existing site conditions, please refer to the provided aerials and existing conditions maps on Sheet EX-1.

2.2. EXISTING SITE DRAINAGE

The current site drainage involves four sub basins according to the historical drainage patterns of the original site. Sub-Basin "A" drains 1.35 acres towards the northeast corner retention area and lowest point on site. This flow causes flooding in that section during significant storm events. Sub-Basin "B" historically drains 1.21 acres to the south draining into two existing inlets connected by exfiltration trench, effectively preventing any flooding during significant storm events. The Sub-Basin "C" drains 0.39 acres westerly into the pervious area on-site. The Sub-Basin "D" drainages the remaining 2.13 acres to three storm inlets, these inlets are not included in this analysis since no changes were made to this area. A Sub-Basin map is provided in Appendix D.

2.3. CONTROL ELEVATION

From the Geotechnical report provided by Andersen Andre Consulting Engineers, Inc. (AACE). It was found that the ground was table was lower then 72 inches from existing grade. This was from hand augers done on 4/12/2022 onsite. These finds are correlated from the USGA Soils Map also estimating the ground water to be deeper then 80 inches. With the average ground elevation for the site at 30.65 ft NVAD and both the estimated water table elevations be deeper the 72 and 80 inches. The estimated control elevation was set as 23.17 ft NVAD.

3. PROPOSED CONDITIONS

3.1. PROJECT DESCRIPTION

The proposed improvements for the site will be to stabilize the old building footprint with an impervious section consisting of 8" shell rock base (LBR 100); and 12" stabilized subgrade (LBR-40) to allow for boat/RV storage, an on-site stormwater drainage area to treat water quality in the northeast corner, and perimeter fencing and landscaping. The water quality treatment volume will be mitigated by the retention area in the East and the exfiltration trench in the middle of the site. See the proposed site plan, on Sheet S-1, for the layout of the future site.

3.2. LAND USE

Table 1 shows a breakdown of the proposed land use compared to the existing land use for the site. The drainage features of the area outside of the stormwater management area will not be altered from existing conditions. This is why the table below 3.83 acres in total compared to the overall 5.08 acres. The pond bottom for the proposed retention area is at elevation 27.0 ft-NAVD.

Table 1 – Land Use Summary

Land Use Summary			
Land Use	Pre-Dev (ac)	Post-Dev (ac)	Difference (ac)
Open-Space	0.56	0.74	0.18
Old Building Space	0.67	0.00	(-)0.67
Existing Asphalt	2.60	2.42	(-)0.18
Shell Rock Base	0.00	0.67	0.67
Parcel Total	3.83	3.83	

3.3. WATER QUALITY

Water quality storage is provided for the site based on the greater of 1 inch of rainfall over the project area or the total runoff of 2.5 inches times the percent impervious. Net impervious area was assumed to be inclusive of the existing pavement area, the gravel storage area, and the open space. The required 0.32 ac-ft of storage will be provided in the proposed retention pond based on the 1-inch over project area. Calculations are provided in Appendix C.

3.4. STORMWATER RUNOFF VOLUME

The proposed site conditions have a net neutral generation of stormwater runoff. The following design storms were analyzed.

- The minimum road crown elevation will be above the 5-yr / 1-day.
- The 100-year / 1-day storm was used to review pre vs. post discharge and used for the retention recovery analysis.

Table 1 – Pre vs. Post Runoff Volume Summary

Runoff Volume			
Design Storm	Pre-Development (ac-ft)	Post-Development (ac-ft)	Difference (ac-ft)
5-yr / 1-day	1.45	1.45	0.00
25-yr / 3-day	3.11	3.11	0.00
100-yr / 1-day	3.36	3.36	0.00

Proposed stormwater calculations are provided in Appendix C.

3.5. STORMWATER DESIGN ELEVATION

The proposed dry retention area is sized to contain the required water quality volume for the site. The storage area will be set at or above the peak stage from the 100-year / 1-day storm event. The existing roadway/parking area are set above the 5-year / 1-day. Table 2 below provides a summary of the design stage for this project. Proposed stormwater calculations are provided in Appendix C.

Table 2 – Design Elevation Summary

Design Elevations			
Design Storm	Post-Development (ft NAVD)	Design Elev (ft NAVD)	Category
Water Quality	29.43	33.00	WQ
5-yr / 1-day	30.41	33.30	Road
100-yr / 1-day	31.35	31.40	Stored Elevation

Additional project design information is listed as follows:

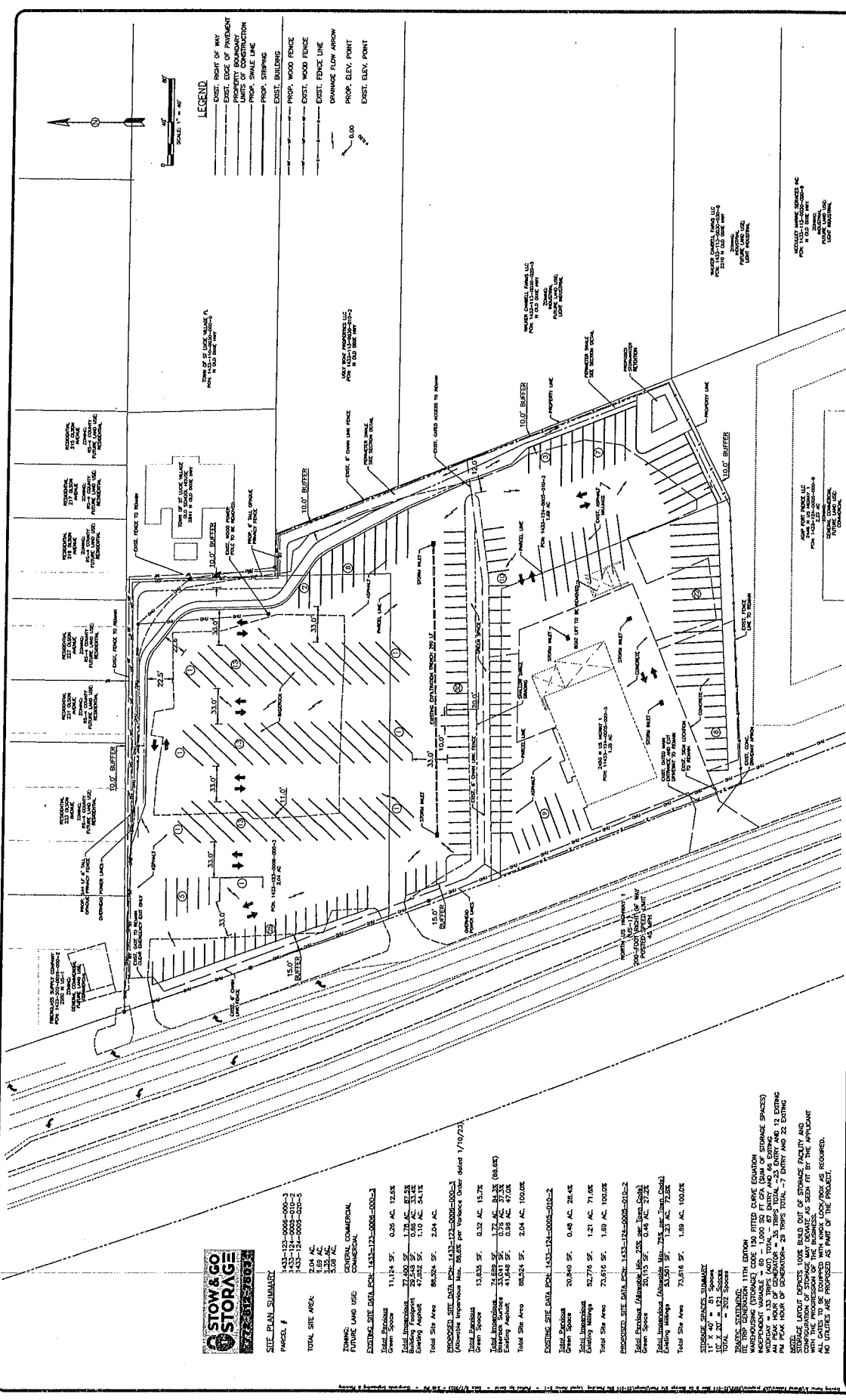
- Rainfall: The design storm for this analysis was the 100-year, 24-hour and 5-year, 24-hour rainfall event was estimated to be 12.0 inches and 5.9 inches per Appendix C of SFWMD's Environment Resource Permit Applicant's Handbook Volume II.

- Outfall: There is no outfall on site, as the site will fully contain the runoff up to the 100 year / 1-day storm event.
- Recovery: The dry retention pond must recover half of its treatment volume in less than five days. In addition, the pond must recover 90% of the 25-year, 3-day runoff volume (evaluated based on the 100-year, 1-day runoff volume) within 12 days from the cessation of the storm event. The 50% treatment volume and 90% runoff volume are recovered 0.061 days and 0.16 days, respectively. The retention area's recovery calculations are provided with the post development calculations in Appendix C.

Geotechnical Information: The soils onsite were evaluated by Andersen Andre Consulting Engineers, Inc. The soils were found to be a mixture of poorly graded sands ranging from dark gray to light brown, with trace amounts of rock fragments found within the 0 to 32 inch range. Orange-brown poorly graded sands with a trace amount of clay was found within the 10 to 72 inch range, sampling was terminated at 72 inches.

APPENDICES

Appendix A
Site Plan



SITE PLAN SUMMARY

PARCELS #	1431-123-000-000-2	1431-123-000-010-2	1431-123-000-010-3	1431-123-000-010-4	1431-123-000-010-5
TOTAL SITE AREA	1.09 AC.	1.09 AC.	1.09 AC.	1.09 AC.	1.09 AC.
TOWN'S FUTURE LAND USE	COMMERCIAL	COMMERCIAL	COMMERCIAL	COMMERCIAL	COMMERCIAL
EXISTING SITE DATA	<p>Green Space: 11,124 SF, 0.26 AC, 12.6%</p> <p>Total Impervious: 27,600 SF, 1.28 AC, 61.4%</p> <p>Building Footprint: 22,549 SF, 0.88 AC, 51.4%</p> <p>Existing Asphalt: 47,352 SF, 1.10 AC, 54.1%</p> <p>Total Site Area: 86,324 SF, 2.04 AC.</p>				
PROPOSED SITE DATA	<p>Green Space: 13,820 SF, 0.32 AC, 15.7%</p> <p>Total Impervious: 31,693 SF, 1.37 AC, 68.3%</p> <p>Building Footprint: 1,000,000 SF, 41.84 AC, 47.0%</p> <p>Existing Asphalt: 41,849 SF, 0.98 AC, 47.0%</p> <p>Total Site Area: 88,524 SF, 2.04 AC, 100.0%</p>				
EXISTING SITE DATA	<p>Total Building: 20,840 SF, 0.48 AC, 26.4%</p> <p>Total Impervious: 32,776 SF, 1.21 AC, 71.0%</p> <p>Total Site Area: 73,616 SF, 1.69 AC, 100.0%</p>				
PROPOSED SITE DATA	<p>Total Building: 1,000,000 SF, 41.84 AC, 100.0%</p> <p>Total Impervious: 1,000,000 SF, 41.84 AC, 100.0%</p> <p>Total Site Area: 73,616 SF, 1.69 AC, 100.0%</p>				

STORAGE SEATS SUMMARY

11' X 20' = 81 Seats
 11' X 10' = 81 Seats
 TOTAL SEATS = 202 SEATS

TRUCK STORAGE

TRUCK STORAGE (STANDARD) 10' X 20' FITTED CARIC EQUIPMENT
 INDEPENDENT VARIABLE = 60' - 1,000 SQ FT CAR (SUM OF STORAGE SPACES)
 ALL PEAK HOUR OF CONCENTRATION = 35 TRIPS TOTAL = 3 ENTRY AND 12 EXITING
 ALL PEAK HOUR OF CONCENTRATION = 29 TRIPS TOTAL = 7 ENTRY AND 22 EXITING

STORAGE LAYOUT DEPICTS 100% BUILD OUT OF STORAGE FACILITY AND
 OPERATIONAL OF STORAGE MAY VARY AS SEEN AS SHOWN BY THE APPLICANT
 ALL GATES TO BE EQUIPPED WITH KNOB LOCK/BOX AS REQUIRED.
 ALL UTILITIES ARE PROPOSED AS PART OF THE PROJECT.

PROJECT # 22-014
S-1

FINAL SITE PLAN

MEGHAN AND BRANDON HAYNES
STOW & GO STORAGE
TOWN OF ST LUCIE VILLAGE, FLORIDA

GRANDGRANDE ENGINEERING AND PLANNING
 GRANDGRANDE ENGINEERING AND PLANNING
 1000 W. UNIVERSITY BLVD.
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 PH: (772) 888-8078
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4. 21-14-0000, PZ
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 gdp@grandgrande.com

Ced: No. 20091
 License No. 68232

DATE	BY	REVISIONS

Appendix B
Stormwater Calculations – Existing Conditions



**Pre-Development Land Use Calculations
Project Explanation**

Project Name: Stow & Go Storage
 Project #: 22-014
 Engineer: DCC
 Date: 8/23/2022

Engineer:
 Revision Date:

Computation Type: Land Use & Stage-Storage
 Datum: NAVD

Average Ground Elevation (EL_{grnd}) = 30.65 ft-NAVD Source: USDA Soil Map
 Control Elevation (CE) = 23.17 ft-NAVD

Site Land Use

	Green Space	Compacted Cogulna	Existing Asphalt	Exist. Area 4	Exist. Area 5	Total
	(ac)	(ac)	(ac)	(ac)	(ac)	(ac)
Percentage of On-Site	20.1%	19.8%	60.1%	0.00%	0.00%	
Total Areas (SF)	33,637 SF	33,041 SF	100,333 SF	0 SF	0 SF	167,011 SF
Total Areas (A_t)	0.77	0.76	2.30	0.00	0.00	3.83
Bldg. %	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%
Bldg. Area (A_b)	0.00	0.00	0.00	0.00	0.00	0.00
Pervious %	100.00%	0.00%	0.00%	0.00%	0.00%	20.14%
Pervious Area (A_p)	0.77	0.00	0.00	0.00	0.00	0.77
Impervious %	0.00%	100.00%	100.00%	0.00%	0.00%	79.86%
Impervious Area (A_i)	0.00	0.76	2.30	0.00	0.00	3.06
Open Water %	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%
Open Water Area (Ac.)	0.00	0.00	0.00	0.00	0.00	0.00

Pre-Development Stage-Storage Calculations
Project Explanation

Storage Type: Stage-Volume
Starting Stage = 27.00
Ending Stage = 34.00
Stage Increment = 0.10

Name	Green Space	Compacted Coquina	Existing Asphalt	
Area	0.77	0.76	2.30	
Start Elev	28.50	29.07	28.57	
End Elev	32.80	31.61	33.30	
Stage	Linear	Linear	Linear	Total
Feet	Storage	Storage	Storage	Storage
NAVD	Ac-ft	Ac-ft	Ac-ft	Ac-ft
27.00	0.00	0.00	0.00	0.00
27.10	0.00	0.00	0.00	0.00
27.20	0.00	0.00	0.00	0.00
27.30	0.00	0.00	0.00	0.00
27.40	0.00	0.00	0.00	0.00
27.50	0.00	0.00	0.00	0.00
27.60	0.00	0.00	0.00	0.00
27.70	0.00	0.00	0.00	0.00
27.80	0.00	0.00	0.00	0.00
27.90	0.00	0.00	0.00	0.00
28.00	0.00	0.00	0.00	0.00
28.10	0.00	0.00	0.00	0.00
28.20	0.00	0.00	0.00	0.00
28.30	0.00	0.00	0.00	0.00
28.40	0.00	0.00	0.00	0.00
28.50	0.00	0.00	0.00	0.00
28.60	0.00	0.00	0.00	0.00
28.70	0.00	0.00	0.00	0.01
28.80	0.01	0.00	0.01	0.02
28.90	0.01	0.00	0.03	0.04
29.00	0.02	0.00	0.05	0.07
29.10	0.03	0.00	0.07	0.10
29.20	0.04	0.00	0.10	0.14
29.30	0.06	0.01	0.13	0.20
29.40	0.07	0.02	0.17	0.26
29.50	0.09	0.03	0.21	0.33
29.60	0.11	0.04	0.26	0.41
29.70	0.13	0.06	0.31	0.50
29.80	0.15	0.08	0.37	0.60
29.90	0.18	0.10	0.43	0.71
30.00	0.20	0.13	0.50	0.83
30.10	0.23	0.16	0.57	0.96
30.20	0.26	0.19	0.65	1.10
30.30	0.29	0.23	0.73	1.25
30.40	0.32	0.26	0.82	1.40
30.50	0.36	0.31	0.91	1.57

**Pre-Development Stage-Area Calculations
Project Explanation**

Storage Type: Stage-Area
Starting Stage = 3.00
Ending Stage = 6.50
Stage Increment = 0.10

Name	Green Space	Compacted Coquina	Existing Asphalt	
Area	0.77	0.76	2.30	
Start Elev	28.50	29.07	28.57	
End Elev	32.80	31.61	33.30	
Stage	Linear	Linear	Linear	Total
Feet	Area	Area	Area	Area
NAVD	Ac	Ac	Ac	Ac
3.00	0.00	0.00	0.00	0.00
3.10	0.00	0.00	0.00	0.00
3.20	0.00	0.00	0.00	0.00
3.30	0.00	0.00	0.00	0.00
3.40	0.00	0.00	0.00	0.00
3.50	0.00	0.00	0.00	0.00
3.60	0.00	0.00	0.00	0.00
3.70	0.00	0.00	0.00	0.00
3.80	0.00	0.00	0.00	0.00
3.90	0.00	0.00	0.00	0.00
4.00	0.00	0.00	0.00	0.00
4.10	0.00	0.00	0.00	0.00
4.20	0.00	0.00	0.00	0.00
4.30	0.00	0.00	0.00	0.00
4.40	0.00	0.00	0.00	0.00
4.50	0.00	0.00	0.00	0.00
4.60	0.00	0.00	0.00	0.00
4.70	0.00	0.00	0.00	0.00
4.80	0.00	0.00	0.00	0.00
4.90	0.00	0.00	0.00	0.00
5.00	0.00	0.00	0.00	0.00
5.10	0.00	0.00	0.00	0.00
5.20	0.00	0.00	0.00	0.00
5.30	0.00	0.00	0.00	0.00
5.40	0.00	0.00	0.00	0.00
5.50	0.00	0.00	0.00	0.00
5.60	0.00	0.00	0.00	0.00
5.70	0.00	0.00	0.00	0.00
5.80	0.00	0.00	0.00	0.00
5.90	0.00	0.00	0.00	0.00
6.00	0.00	0.00	0.00	0.00
6.10	0.00	0.00	0.00	0.00
6.20	0.00	0.00	0.00	0.00
6.30	0.00	0.00	0.00	0.00
6.40	0.00	0.00	0.00	0.00
6.50	0.00	0.00	0.00	0.00

PRE-Development Stormwater Calculations

Project Explanation

Project: Stow & Go Storage
Project #: 22-014

Revised: _____
Engineer: DCC
Date: 8/23/2022

Land Use Table

Use	Imp. (ac)	Bldg. (ac)	Pervious (ac)	Total (ac)
Basin =	3.06	0.00	0.77	3.83
TOTAL =	3.06	0.00	0.77	3.83

Find Curve Number:

Avg. Pervious Ground El. =	30.65
Control Elev. =	23.17
Distance to Water Table =	7.48

Soil Storage Table

Depth to W.T. (ft)	Coastal Storage (in)	Flatwoods Storage (in)	Depression Storage (in)
0.0	0.0	0.0	0.0
1.0	0.6	0.6	0.6
1.5	1.6	1.6	1.4
2.0	2.5	2.5	2.1
2.5	4.6	4.0	3.3
3.0	6.6	5.4	4.4
3.5	8.8	7.2	5.6
4.0	10.9	9.0	6.8

Find the basin's soil classification and input below as "Soil Storage" using the above "Depth to W.T.".

Soil Type =	Flatwoods	
Max. Available Soil Storage (S_{max}) =	9.00	inches
Compaction Factor ($F_{compact}$) =	25%	(Use 25% for developed site)
Compacted Soil Storage ($S_{compact}$) =	6.75	inches
Available Soil Storage (S_{avail}) =	1.36	inches
Curve Number (CN) =	88	$1000 / (S_{avail} + 10)$

Notes:

1. Soil Storage Table taken from Section 5.7.4.2 of the SFWMD ERP Applicant's Handbook Volume II.
2. Curve Number determination based on method presented in USDA NRCS Technical Release 55 (TR-55) "Urban Hydrology for Small Watersheds".

Appendix C
Stormwater Calculations – Proposed Conditions



**Post-Development Land Use Calculations
Project Explanation**

Project Name: **Stow & Go Storage**
 Project #: **22-014**
 Engineer: **DCC**
 Date: **8/23/2022**

Engineer:
 Revision Date:

Computation Type: **Land Use & Stage-Storage**
 Datum: **NAVD**

Average Ground Elevation (EL_{gmd}) = **30.65** ft-NAVD Source: **USDA Soil Map**
 Control Elevation (CE) = **23.17** ft-NAVD

Site Land Use

	Open Space	Compacted Coquina	Existing Asphalt	Retention Pond Bottom	Exist. Area 5	Total
	(ac)	(ac)	(ac)	(ac)	(ac)	(ac)
Percentage of On-Site	17.5%	19.8%	60.1%	2.68%	0.00%	
Total Areas (SF)	29,159 SF	33,041 SF	100,333 SF	4,478 SF	0 SF	167,011 SF
Total Areas (A _i)	0.67	0.76	2.30	0.10	0.00	3.83
Bldg. %	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%
Bldg. Area (A _b)	0.00	0.00	0.00	0.00	0.00	0.00
Pervious %	100.00%	0.00%	0.00%	100.00%	0.00%	20.14%
Pervious Area (A _p)	0.67	0.00	0.00	0.10	0.00	0.77
Impervious %	0.00%	100.00%	100.00%	0.00%	0.00%	79.86%
Impervious Area (A _i)	0.00	0.76	2.30	0.00	0.00	3.06
Open Water %	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%
Open Water Area (Ac.)	0.00	0.00	0.00	0.00	0.00	0.00



Post-Development Stage-Storage Calculations
Project Explanation

Storage Type: Stage-Volume
Starting Stage = 25.00
Ending Stage = 32.60
Stage Increment = 0.10

Name	Open Space	Compacted Coquina	Existing Asphalt	Retention Pond	Exist. Area 8	Exist. Area 9	
Area	0.67	0.76	2.30	0.10	0.00	0.00	
Start Elev	28.50	29.07	28.57	27.00	0.00	0.00	
End Elev	33.00	32.50	33.00	33.00	0.00	0.00	
Stage Feet	Linear Storage Ac-ft	Linear Storage Ac-ft	Linear Storage Ac-ft	Linear Storage Ac-ft	Vert Storage Ac-ft	Vert Storage Ac-ft	Total Storage Ac-ft
NAVD							
25.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
25.10	0.00	0.00	0.00	0.00	0.00	0.00	0.00
25.20	0.00	0.00	0.00	0.00	0.00	0.00	0.00
25.30	0.00	0.00	0.00	0.00	0.00	0.00	0.00
25.40	0.00	0.00	0.00	0.00	0.00	0.00	0.00
25.50	0.00	0.00	0.00	0.00	0.00	0.00	0.00
25.60	0.00	0.00	0.00	0.00	0.00	0.00	0.00
25.70	0.00	0.00	0.00	0.00	0.00	0.00	0.00
25.80	0.00	0.00	0.00	0.00	0.00	0.00	0.00
25.90	0.00	0.00	0.00	0.00	0.00	0.00	0.00
26.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
26.10	0.00	0.00	0.00	0.00	0.00	0.00	0.00
26.20	0.00	0.00	0.00	0.00	0.00	0.00	0.00
26.30	0.00	0.00	0.00	0.00	0.00	0.00	0.00
26.40	0.00	0.00	0.00	0.00	0.00	0.00	0.00
26.50	0.00	0.00	0.00	0.00	0.00	0.00	0.00
26.60	0.00	0.00	0.00	0.00	0.00	0.00	0.00
26.70	0.00	0.00	0.00	0.00	0.00	0.00	0.00
26.80	0.00	0.00	0.00	0.00	0.00	0.00	0.00
26.90	0.00	0.00	0.00	0.00	0.00	0.00	0.00
27.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
27.10	0.00	0.00	0.00	0.00	0.00	0.00	0.00
27.20	0.00	0.00	0.00	0.00	0.00	0.00	0.00
27.30	0.00	0.00	0.00	0.00	0.00	0.00	0.00
27.40	0.00	0.00	0.00	0.00	0.00	0.00	0.00
27.50	0.00	0.00	0.00	0.00	0.00	0.00	0.00
27.60	0.00	0.00	0.00	0.00	0.00	0.00	0.00
27.70	0.00	0.00	0.00	0.00	0.00	0.00	0.00
27.80	0.00	0.00	0.00	0.01	0.00	0.00	0.01
27.90	0.00	0.00	0.00	0.01	0.00	0.00	0.01
28.00	0.00	0.00	0.00	0.01	0.00	0.00	0.01
28.10	0.00	0.00	0.00	0.01	0.00	0.00	0.01
28.20	0.00	0.00	0.00	0.01	0.00	0.00	0.01
28.30	0.00	0.00	0.00	0.01	0.00	0.00	0.01
28.40	0.00	0.00	0.00	0.02	0.00	0.00	0.02
28.50	0.00	0.00	0.00	0.02	0.00	0.00	0.02

Post-Development Stage-Area Calculations
Project Explanation

Storage Type: Stage-Area
 Starting Stage = 3.00
 Ending Stage = 6.50
 Stage Increment = 0.10

Name	Open Space	Compacted Coquina	Existin g	Retention Pond		
Area	0.67	0.76	2.30	0.10		
Start Elev	28.50	29.07	28.57	27.00		
End Elev	33.00	32.50	33.00	33.00		
Stage Feet NAVD	Linear Area Ac	Linear Area Ac	Linear Area Ac	Linear Area Ac	Linear Area Ac	Total Area Ac
3.00	0.00	0.00	0.00	0.00	0.00	0.00
3.10	0.00	0.00	0.00	0.00	0.00	0.00
3.20	0.00	0.00	0.00	0.00	0.00	0.00
3.30	0.00	0.00	0.00	0.00	0.00	0.00
3.40	0.00	0.00	0.00	0.00	0.00	0.00
3.50	0.00	0.00	0.00	0.00	0.00	0.00
3.60	0.00	0.00	0.00	0.00	0.00	0.00
3.70	0.00	0.00	0.00	0.00	0.00	0.00
3.80	0.00	0.00	0.00	0.00	0.00	0.00
3.90	0.00	0.00	0.00	0.00	0.00	0.00
4.00	0.00	0.00	0.00	0.00	0.00	0.00
4.10	0.00	0.00	0.00	0.00	0.00	0.00
4.20	0.00	0.00	0.00	0.00	0.00	0.00
4.30	0.00	0.00	0.00	0.00	0.00	0.00
4.40	0.00	0.00	0.00	0.00	0.00	0.00
4.50	0.00	0.00	0.00	0.00	0.00	0.00
4.60	0.00	0.00	0.00	0.00	0.00	0.00
4.70	0.00	0.00	0.00	0.00	0.00	0.00
4.80	0.00	0.00	0.00	0.00	0.00	0.00
4.90	0.00	0.00	0.00	0.00	0.00	0.00
5.00	0.00	0.00	0.00	0.00	0.00	0.00
5.10	0.00	0.00	0.00	0.00	0.00	0.00
5.20	0.00	0.00	0.00	0.00	0.00	0.00
5.30	0.00	0.00	0.00	0.00	0.00	0.00
5.40	0.00	0.00	0.00	0.00	0.00	0.00
5.50	0.00	0.00	0.00	0.00	0.00	0.00
5.60	0.00	0.00	0.00	0.00	0.00	0.00
5.70	0.00	0.00	0.00	0.00	0.00	0.00
5.80	0.00	0.00	0.00	0.00	0.00	0.00
5.90	0.00	0.00	0.00	0.00	0.00	0.00
6.00	0.00	0.00	0.00	0.00	0.00	0.00
6.10	0.00	0.00	0.00	0.00	0.00	0.00
6.20	0.00	0.00	0.00	0.00	0.00	0.00
6.30	0.00	0.00	0.00	0.00	0.00	0.00
6.40	0.00	0.00	0.00	0.00	0.00	0.00
6.50	0.00	0.00	0.00	0.00	0.00	0.00

Post-Development Stormwater Calculations

Project Explanation

Project: Stow & Go Storage
Project #: 22-014

Revised: _____
Engineer: DCC
Date: 8/23/2022

Land Use Table

Use	Imp. (ac)	Bldg. (ac)	Pervious (ac)	Total (ac)
Basin =	3.06	0.00	0.77	3.83
TOTAL =	3.06	0.00	0.77	3.83

Find Curve Number:

Avg. Pervious Ground El. =	30.65
Control Elev. =	23.17
Distance to Water Table =	7.48

Soil Storage Table

Depth to W.T. (ft)	Coastal Storage (in)	Flatwoods Storage (in)	Depression Storage (in)
0.0	0.0	0.0	0.0
1.0	0.6	0.6	0.6
1.5	1.6	1.6	1.4
2.0	2.5	2.5	2.1
2.5	4.6	4.0	3.3
3.0	6.6	5.4	4.4
3.5	8.8	7.2	5.6
4.0	10.9	9.0	6.8

Find the basin's soil classification and input below as "Soil Storage" using the above "Depth to W.T.".

Soil Type =	Flatwoods	
Max. Available Soil Storage (S_{max}) =	9.00	inches
Compaction Factor ($F_{compact}$) =	25%	(Use 25% for developed site)
Compacted Soil Storage ($S_{compact}$) =	6.75	inches
Available Soil Storage (S_{avail}) =	1.36	inches
Curve Number (CN) =	85	$\frac{(S_{max})(F_{compact})}{(A_p)(S_{compact})} / (A_t)$ $1000 / (S_{avail} + 10)$

Notes:

1. Soil Storage Table taken from Section 5.7.4.2 of the SFWMD ERP Applicant's Handbook Volume II.
2. Curve Number determination based on method presented in USDA NRCS Technical Release 55 (TR-55) "Urban Hydrology for Small Watersheds".



Post-Development Water Quality Calculations

Project: <u>Stow & Go Storage</u>	Revised: _____
Project #: <u>22-014</u>	Engineer: <u>DCC</u>
	Date: <u>23-Aug-22</u>

1-inch Over the Project Area

$$\begin{array}{rcl}
 \text{(Treated Volume)} & 1\text{-inch} & * & 1\text{-ft}/12\text{-in} & * & \frac{3.83}{\text{PROJECT AREA}} & = & \boxed{0.32} & \text{ac-ft} \\
 & & & & & & & & \text{TREATED VOLUME}
 \end{array}$$

2.5-inches Times the Percent Impervious

(Site Area)	<u>3.83</u>	-	<u>0.00</u>	+	<u>0.00</u>)	=	<u>3.83</u>	ac
	PROJECT AREA (AC)		LAKES (AC)		ROOFS (AC)			SITE AREA	
(Impervious Area)	<u>3.83</u>	-	<u>0.77</u>)	=	<u>3.06</u>	ac
	SITE AREA (AC)		PERVIOUS AREA (AC)					IMPERVIOUS AREA	
(% Impervious)			<u>IMPERVIOUS AREA * 100%</u>				=	<u>79.86%</u>	
			SITE AREA (AC)						
(2.5-in * % Imp.)			2.5-inches	*	<u>79.86%</u>			=	<u>2.00</u> in
					PERCENT IMPERVIOUS			INCHES TO BE TREATED	
(Treated Volume)	<u>2.00</u>	*	1-ft/12-in	*	<u>3.83</u>			=	<u>0.64</u> ac-ft
	TREATED (IN)				PROJECT AREA - LAKES (AC)			TREATED VOLUME	

Required Wet Detention = 0.64 ac-ft

Required Dry Detention (75% of Wet Detention) = 0.48 ac-ft

Required Retention (50% of Wet Detention) = 0.32 ac-ft

Treatment System Type	Retention
Treatment Volume Required	= 0.32 ac-ft

Direct Discharge to OFW?	No
Total Treatment Volume Required	= 0.32 ac-ft

Water Quality Elevation (EL_{wq}) = 29.43 ft NAVD

EXFILTRATION TRENCH DESIGN

Project: Stow & Go Storage Engineer: DCC
 Project #: 22-014 Date: 8/23/2022

Water Quality Volume = **0.32 Ac-ft**

$$L = \frac{V}{(K(H_2*W+2*H_2*D_u-D_u^2+2*H_2*D_s)+(1.39*10^{-4})*W*D_u)} \quad \text{(Dry System)}$$

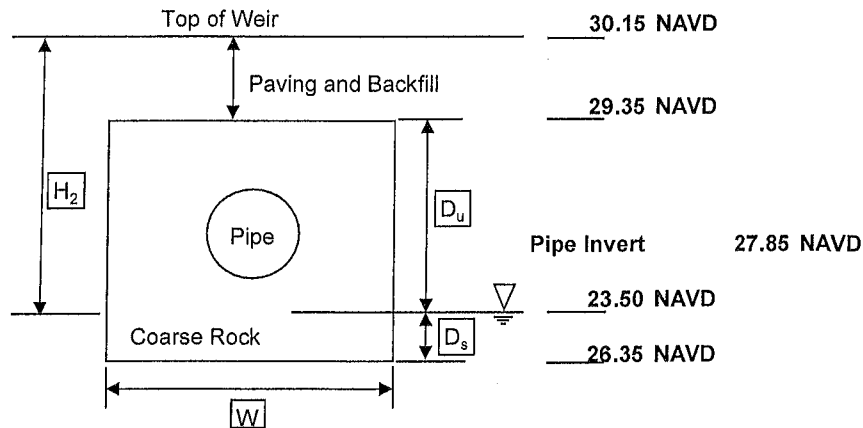
$$L = \frac{V}{(K(2*H_2*D_u-D_u^2+2*H_2*D_s)+(1.39*10^{-4})*W*D_u)} \quad \text{(Wet System)}$$

V	Volume Treated	3.83 Ac-in
W	Trench Width	3.00 ft
K	Hydraulic Conductivity *	6.55E-04 cfs/ft ² -ft.head
H ₂	Depth to Water Table	6.65 ft
D _u	Non Saturated Trench Depth	3.00 ft
D _s	Saturated Trench Depth (feet)	0.00 ft
L	Length of Trench Required	111 ft

$$V = L*(K(H_2*W+2*H_2*D_u-D_u^2+2*H_2*D_s)+(1.39*10^{-4})*W*D_u) \quad \text{(Dry System)}$$

$$V = L*(K(2*H_2*D_u-D_u^2+2*H_2*D_s)+(1.39*10^{-4})*W*D_u) \quad \text{(Wet System)}$$

L	Length of Trench Provided	260 ft
W	Trench Width	3.00 ft
K	Hydraulic Conductivity (cfs/ft ² -ft.head)	6.55E-04
H ₂	Depth to Water Table	6.65 ft
D _u	Non Saturated Trench Depth	3.00 ft
D _s	Saturated Trench Depth	0.00 ft
V	Volume Treated	8.99 Ac-in 0.749 Ac-ft



* Hydraulic Conductivity based on AACE Geotechnical Investigation, file No. 22-245



Estimated Required Attenuation
Project Explanation

Project: Stow & Go Storage Revised: _____
Project #: 22-014 Engineer: DCC
Date: 8/23/2022

Pre-Developed Runoff

5 Year - 1 Day

Area, A (ac) = 3.83
Soil Storage, S (inches) = 1.36
Curve Number, CN = $1000/(S+10)$
= 88

5 Year - 1 Day Rainfall, P (inches) = 5.9 (From Figure C-8 SFWMD ERP Applicant's Handbook Volume II)

Depth of Runoff, Q (inches) = $(P - 0.2S)^2/(P + 0.8S)$
= 4.53

Volume, V (AC-FT) = A * Q
= 1.45

Post Developed Runoff

5 Year - 1 Day

Area, A (ac) = 3.83
Soil Storage, S (inches) = 1.36
Curve Number, CN = $1000/(S+10)$ =
85

5 Year - 1 Day Rainfall, P (inches) = 5.9 (From Figure C-8 SFWMD ERP Applicant's Handbook Volume II)

Depth of Runoff, Q (inches) = $(P - 0.2S)^2/(P + 0.8S)$
= 4.53

Volume, V (AC-FT) = A * Q
= 1.45

5Y-1D Elevation = 30.41 ft NAVD



Estimated Required Attenuation
Project Explanation

Project: Stow & Go Storage Revised: _____
Project #: 22-014 Engineer: DCC
Date: 8/23/2022

Pre-Developed Runoff

100 Year - 1 Day

Area, A (ac) = 3.83
Soil Storage, S (inches) = 1.36
Curve Number, CN = $1000/(S+10)$
= 88

100 Year - 1 Day Rainfall, P (inches) = 12 (From Figure C-8 SFWMD ERP Applicant's Handbook Volume II)

Depth of Runoff, Q (inches) = $(P - 0.2S)^2/(P + 0.8S)$
= 10.51

Volume, V (AC-FT) = A * Q
= 3.36

Post Developed Runoff

100 Year - 1 Day

Area, A (ac) = 3.83
Soil Storage, S (inches) = 1.36
Curve Number, CN = $1000/(S+10)$ = 31
85

100 Year - 1 Day Rainfall, P (inches) = 12 (From Figure C-8 SFWMD ERP Applicant's Handbook Volume II)

Depth of Runoff, Q (inches) = $(P - 0.2S)^2/(P + 0.8S)$
= 10.51

Volume, V (AC-FT) = A * Q
= 3.36

100Y-1D Elevation = 31.35 ft NAVD

Pond Recovery Calculations

Methodology for Estimating Vertical Unsaturated Flow -- Modified Green and Ampt Infiltration
 Methodology for Estimating Lateral Saturated Flow -- Andreyev and Wiseman (1989)

Project: Stow & Go Storage
 Project #: 22-014

Engineer: DCC
 Date: 6/5/2023

Project Area =	3.83	acres		
% Impervious =	79.86%			
Off-site Drainage Area =	0	acres		
Pond Bottom Elevation =	27.00	FT-NAVD		
Wet Season Water Table =	23.17	FT-NAVD		
Impervious Layer Elevation =	0.00	FT-NAVD		
100y/1d Storage Volume =	3.358	ac-ft		
=	146,271	ft ³	at EL	31.25 FT-NAVD
90% of 100y/1d Storage Volume =	3.022	ac-ft		
=	131,644	ft ³	at EL	31.26 FT-NAVD
50% of 100y/1d Storage Treatment Volume =	1.679	ac-ft		
=	73,136	ft ³	at EL	31.26 FT-NAVD
Fillable porosity (f) =	0.3			
Factor of Safety (FS) =	2			
Hydraulic Conductivity (K) =	6.55E-04	cfs/ft ² -ft.head		
=	56.59	ft/day		
Pond Bottom Area (A _b) =	0.10	acres		
=	4356.0	ft ²	Assume a rect. Pond	
			L =	350.0 ft
			W =	17.5 ft

Time to Recover Half of Treatment Volume:

Height of water above pond bottom (h _v) =	4.26	ft
Height of pond bottom above WSWT (h _b) =	3.83	ft
Height of water reqd to saturate soil (h _u) =	1.149	ft
Unsaturated Vol. (V _u) = A _b h _b f =	5,005	ft ³

Is Treatment Volume ≤ V_u? **NO**
 Is h_v ≤ h_u? **NO**

If either of the above answers is YES, then infiltration is entirely under vertical unsaturated flow, otherwise it also is under saturated lateral flow

Vertical Unsaturated Flow

Unsaturated K = 2/3*K =	37.73	ft/day
Infiltration Rate (I _d) = K/2 =	18.86	ft/day
time to saturation (t _{sat}) =	0.061	days

< 5 days, so OK

Time to Recover 90% of Volume:

Height of water above pond bottom (h_v) = 4.26 ft
Height of pond bottom above WSWT (h_b) = 3.83 ft
Height of water reqd to saturate soil (h_u) = 1.149 ft
Unsaturated Vol. (V_u) = $A_b h_b f$ = 5,005 ft³

Is Treatment Volume $\leq V_u$? **NO**

Is $h_v \leq h_u$? **NO**

If either of the above answers is YES, then infiltration is entirely under vertical unsaturated flow, otherwise it also is under saturated lateral flow

Vertical Unsaturated Flow

Unsaturated K = $2/3 * K$ = 37.73 ft/day
Infiltration Rate (I_d) = $K/2$ = 18.86 ft/day
time to saturation (t_{sat}) = 0.061 days
Remaining Volume = 126,639 ft³
= 2.907 ac-ft at EL 31.20 FT-NAVD

Saturated Lateral Flow

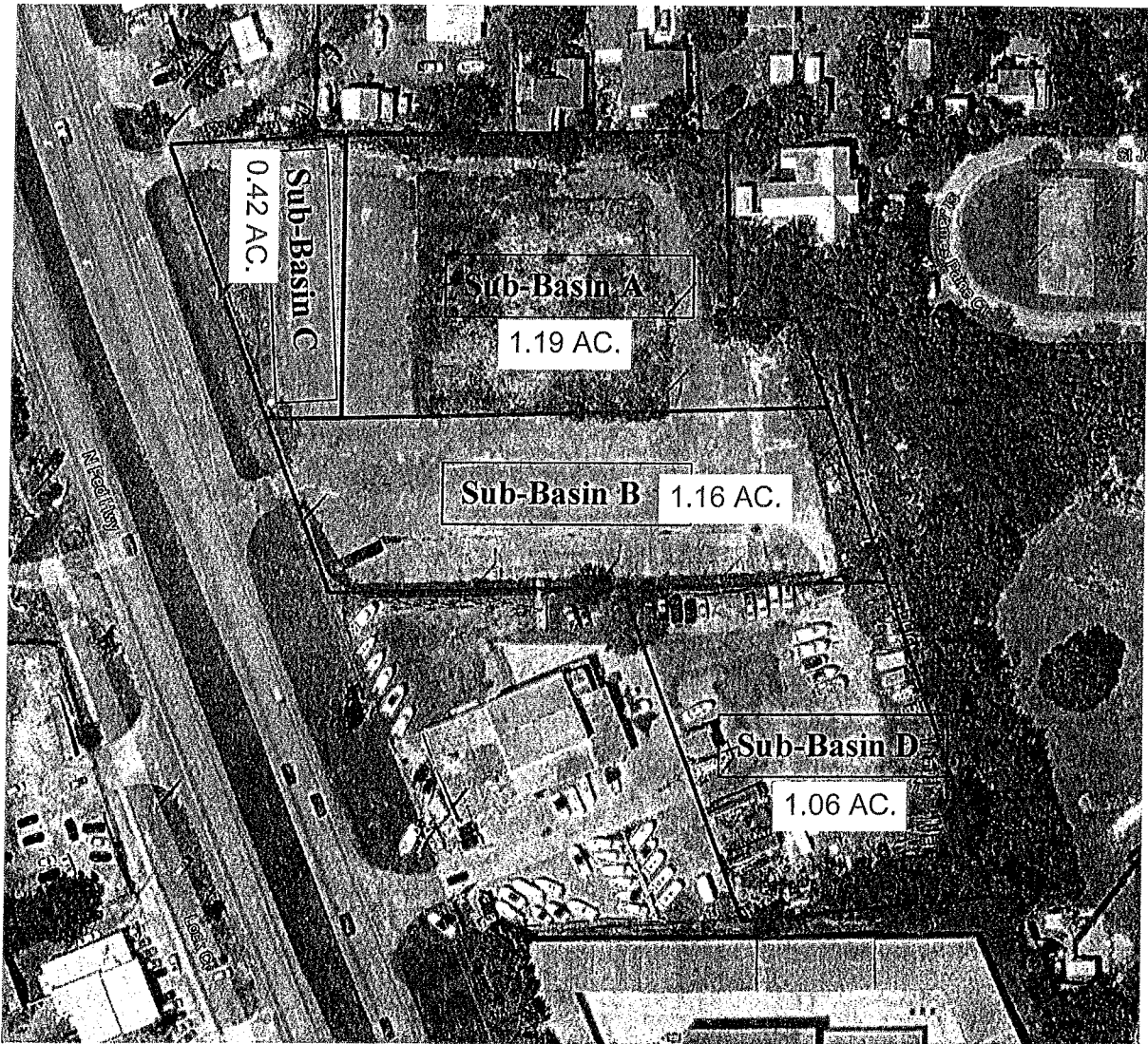
Height of water above WSWT at time t (h_c) = 3.83 ft (at $t = t_{total}$, $h_c = h_b$)
Height above pond bottom for remaining vol. (h_2) = 4.20 ft
 $H_T = h_2 + h_b$ = 8.03 ft
 $F_Y = h_c / H_T$ = 0.48
L:W ratio = L/W = 20.00
therefore, F_x = 0.75

Initial saturated thickness of aquifer (H) = 23.17 ft
Avg saturated thickness of aquifer (D) = $H + h_c/2$ = 25.09 ft
 $t = W^2 / 4 * K * D * F_x^2$ = 0.10 days
 $t_{total} = t_{sat} + t =$ 0.16 days < 12 days, so OK

Notes:

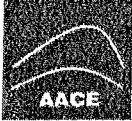
1. Impervious layer elevation assumed from USGS Water Resources Investigations Report 03-4242
2. Fillable porosity (f) assumed based on typical value for sandy soils
3. F_x from Dimensionless Curves Relating Basin Design Parameters to Basin Water Level in a Rectangular Basin over an Unconfined Aquifer (f=0.3) (Source: Andreyev and Wiseman, 1989)

Appendix D
Sub-Basin Map



SUB-BASIN
A = 1.19 AC.
B = 1.16 AC.
C = 0.42 AC.
D = 1.06 AC.
3.83 AC.

Appendix E
Geotechnical Report
(Provided by: Andersen Andre Consulting Engineers, Inc.)



Giangrande Engineering & Planning, Inc.
 22081 SE Ocean Boulevard, Suite 1A
 Stuart, FL 34996

Attention: Mr. Dylan O’Berry, P.E.

SUMMARY REPORT
GEOTECHNICAL ENGINEERING CONSULTING
STOW & GO | 2500 NORTH US HIGHWAY ONE
FORT PIERCE, ST. LUCIE COUNTY, FLORIDA

INTRODUCTION

In accordance with your request and authorization, Andersen Andre Consulting Engineers, Inc. (AACE) has completed the scope of work outlined in our Proposal No. P22-2272 (dated 04/12/22). This report documents our field work and presents our findings.

PROJECT UNDERSTANDING

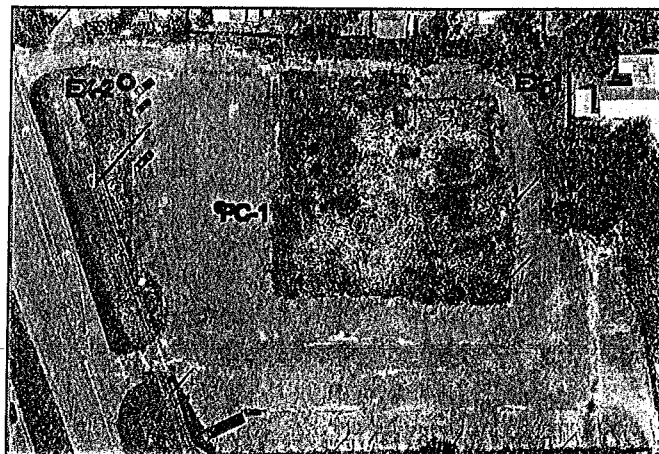
The subject site is located at 2500 North US Highway One in Fort Pierce, St. Lucie County, Florida. Based on our review of the provided project-related information, we understand that it is proposed to improve the site for the purpose of constructing a boat storage yard. We further understand that no Geotechnical Engineering services are being requested.

FIELD EXPLORATION PROGRAM

To explore the site conditions, the following requested exploration program was completed:

Field Exploration Program				
Field Work Type	Standard	# of Borings/Tests	Depth Below Grade [feet]	Location
Pavement Core and Auger Boring (PC-#)	ASTM D1452	1	4	See Below
Soil Hydraulic Conductivity Test (EX-#)	SFWMD ERPIM ⁽¹⁾	2	6	See Below

(1) SFWMD Environmental Resource Permit Information Manual



Our field exploration program was completed August 5, 2022. The shown field work locations were determined in the field by our field crew using the provided schematic, aerial photographs, existing site features, and tape/wheel measurements. The locations should be considered accurate only to the degree implied by the method of measurement used.

FINDINGS

Soil Hydraulic Conductivity Tests

The soil hydraulic conductivity (exfiltration) tests were performed in accordance with methods described in the South Florida Water Management District (SFWMD) Permit Information Manual.

In summary, the exfiltration tests yielded the following results:

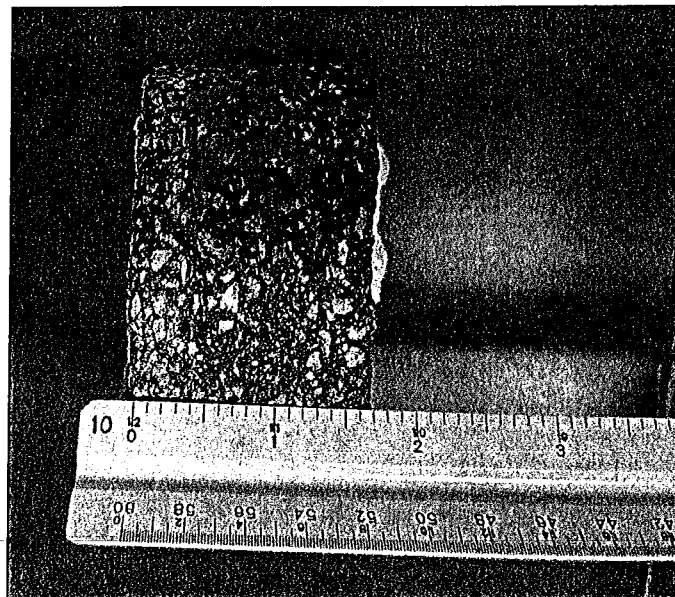
- EX-1; $K = 7.4 \times 10^{-4}$ cfs/ft²-ft. head
- EX-2; $K = 5.7 \times 10^{-4}$ cfs/ft²-ft. head

The exfiltration test reports are attached hereto.

Pavement Core

The pavement coring was performed using a 4-inch diameter core barrel and a hand (bucket) auger so as to determine the thicknesses of the various elements in the existing pavement section (i.e. asphaltic wearing surface, base course and subgrade soils) to a depth of 4 feet below the asphalt surface. Following the coring and hand augering, the borehole was backfilled with accumulated soil cuttings and the core hole was filled with asphaltic "cold-patch" material.

In brief, the pavement core (PC-1) encountered 1¾ inches of asphalt over 8¾ inches of light brown coquina rock, in turn followed by light brown fine sand (AASHTO 'A-3') to a depth of 4 feet below grade. A photograph of the recovered asphalt core is presented below.



CLOSURE

We are pleased to be of assistance to you on this phase of your project. When we may be of further service to you or should you have any questions, please contact us.

Sincerely,
ANDERSEN ANDRE CONSULTING ENGINEERS, INC.



Peter G. Andersen, P.E.
Principal Engineer
Fla. Reg. No. 57956

This report has been digitally signed by Peter G. Andersen, P.E. on the date adjacent to the seal. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.



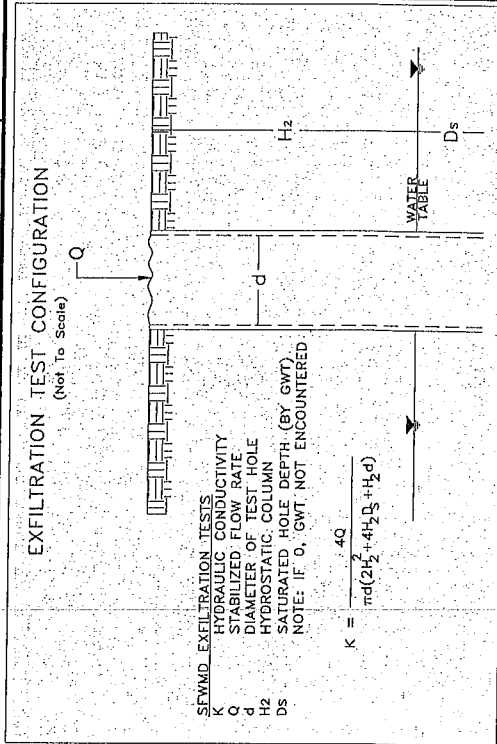


ANDERSEN ANDRE CONSULTING ENGINEERS, INC.

SFWMD Usual Open Hole Test

Test Number	EX-1	Project Name	Stow and Go 2500 N. US Hwy One	Weather Conditions	Pt. Cloudy
		Project Number	22-245	Temperature	91F
		Test Location	NEC of Site	Technician	RL/TM
		Date	08/05/2022	Engineer	PA

DIAMETER OF TEST HOLE (FEET): $d =$	0.5
DEPTH OF TEST HOLE (FEET): $H_1 =$	6
DEPTH TO WATER TABLE (FEET): $H_2 =$	6
SATURATED HOLE DEPTH (FEET): $D_s =$	0
METER READING (Gallons): $V_1 =$	0
METER READING (Gallons): $V_2 =$	98
AVERAGE FLOW RATE (GPM): $Q =$	9.80
"STABILIZED" FLOW RATE (CFS): $Q =$	2.2E-02
HYDRAULIC CONDUCTIVITY (CFS / FT ² - FT. HEAD): $K =$	7.4E-04



Soil Profile	
Depth (in-bls)	Description
0 - 10	Dark gray fine sand (SP), t/o rock fragments
10 - 72	Orange-brown fine sand (SP)
Groundwater not encountered	

NOTES:
 The hydraulic conductivity test was performed in general accordance with the methods described in the South Florida Water Management District (SFWMD) Environmental Resource Permit Information Manual (Volume IV).
 The K-value was calculated based on the exfiltration test procedure as shown hereon.
 The presented hydraulic conductivity (K) value is applicable for an exfiltration trench installed at the same depth as the borehole test. The K-value represents an ultimate value. The designer should decide on the required factor of safety (minimum of 2, per SFWMD).

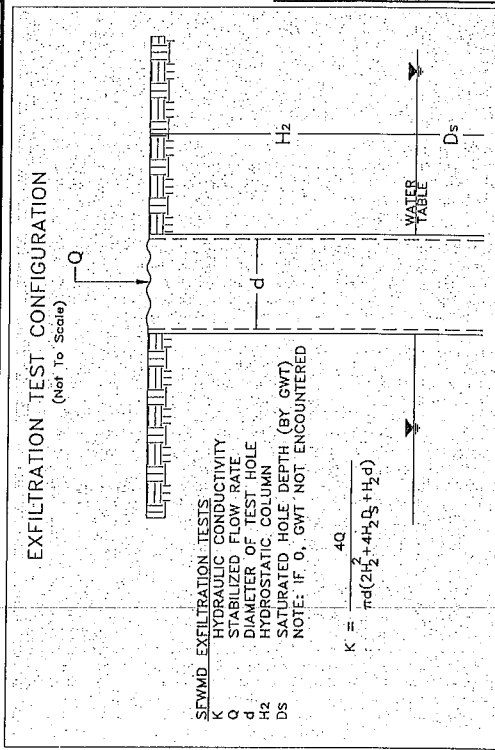


ANDERSEN ANDRE CONSULTING ENGINEERS, INC.

SFWMD Usual Open Hole Test

Test Number	EX-2	Project Name	Stow and Go 2500 N. US Hwy One	Weather Conditions	Pt. Cloudy
		Project Number	22-245	Temperature	90F
		Test Location	NEC of Site	Technician	RL/TM
		Date	08/05/2022	Engineer	PA

DIAMETER OF TEST HOLE (FEET): $d =$	0.5
DEPTH OF TEST HOLE (FEET): $H_1 =$	6
DEPTH TO WATER TABLE (FEET): $H_2 =$	6
SATURATED HOLE DEPTH (FEET): $D_s =$	0
METER READING (Gallons): $V_1 =$	0 @ 0.00 MIN
METER READING (Gallons): $V_f =$	75 @ 10.00 MIN
AVERAGE FLOW RATE (GPM) =	7.50
"STABILIZED" FLOW RATE (CFS): $Q =$	1.7E-02
HYDRAULIC CONDUCTIVITY (CFS / FT ² - FT. HEAD): $K =$	5.7E-04



Depth (in-bl)	Description
0 - 32	Gray-brown fine sand (SP) with rock fragments
32 - 63	Light brown fine sand (SP)
63 - 72	Orange-brown fine sand (SP), t/o clay

NOTES:
 The hydraulic conductivity test was performed in general accordance with the methods described in the South Florida Water Management District (SFWMD) Environmental Resource Permit Information Manual (Volume IV).
 The K-value was calculated based on the exfiltration test procedure as shown hereon.
 The presented hydraulic conductivity (K) value is applicable for an exfiltration trench installed at the same depth as the borehole test. The K-value represents an ultimate value. The designer should decide on the required factor of safety (minimum of 2, per SFWMD).